

\* SURVEY BASICS Made EASY? \*

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\* QUESTIONS, COMMENTS, CRITICISMS & CORRECTIONS WELCOME. \*

## BASIC GENERAL Information

80 CHAINS = 1 MILE = 5,280 Ft.

1 CHAIN = 100 LINKS = 66 Ft.

1 LINK = 7.92 inches = .66 Ft.

10 SQUARE CHAINS = 1 ACRE = 43,560 FT. SQUARED

1 MILE SQUARED = 640 ACRES = 1 SECTION

1 METER = 3.28083 Ft. (3937/1200)

6 SECTIONS x 6 SECTIONS = Township.

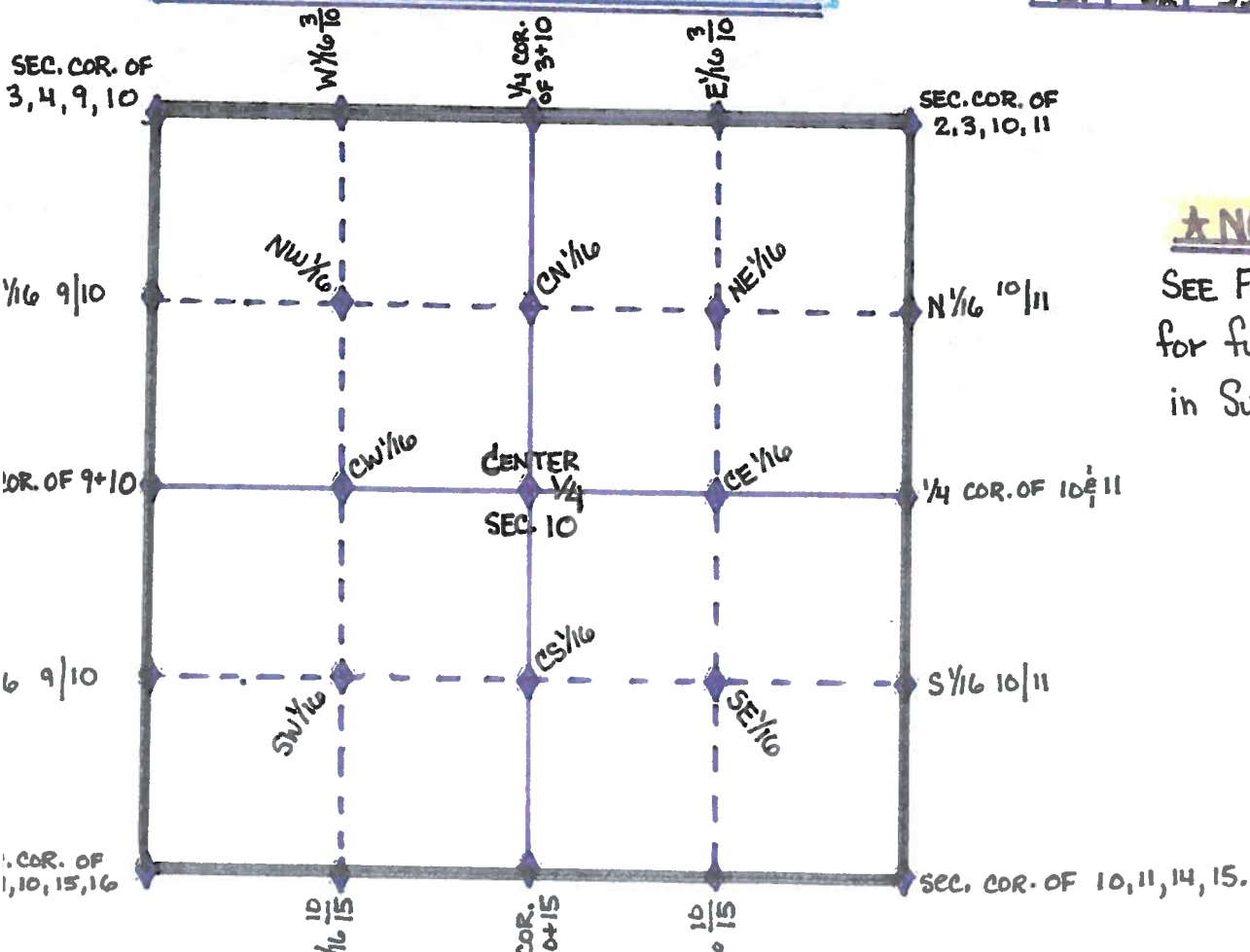
Sections in a township begin with #1 in the NE, Ending with #36 in the SE.

\* BE AWARE ALSO OF THE GCSDB (Geographic COORDINATE DATA BASE) SYSTEM OF NAMING CORNERS. SEE EXPLANATION PG. 38. \*

TYPICAL TOWNSHIP

6	5	4	3	2	1
7	8	9	10	11	12
18	17	16	15	14	13
19	20	21	22	23	24
30	29	28	27	26	25
31	32	33	34	35	36

### NAMES OF CORNERS in Sec. 10

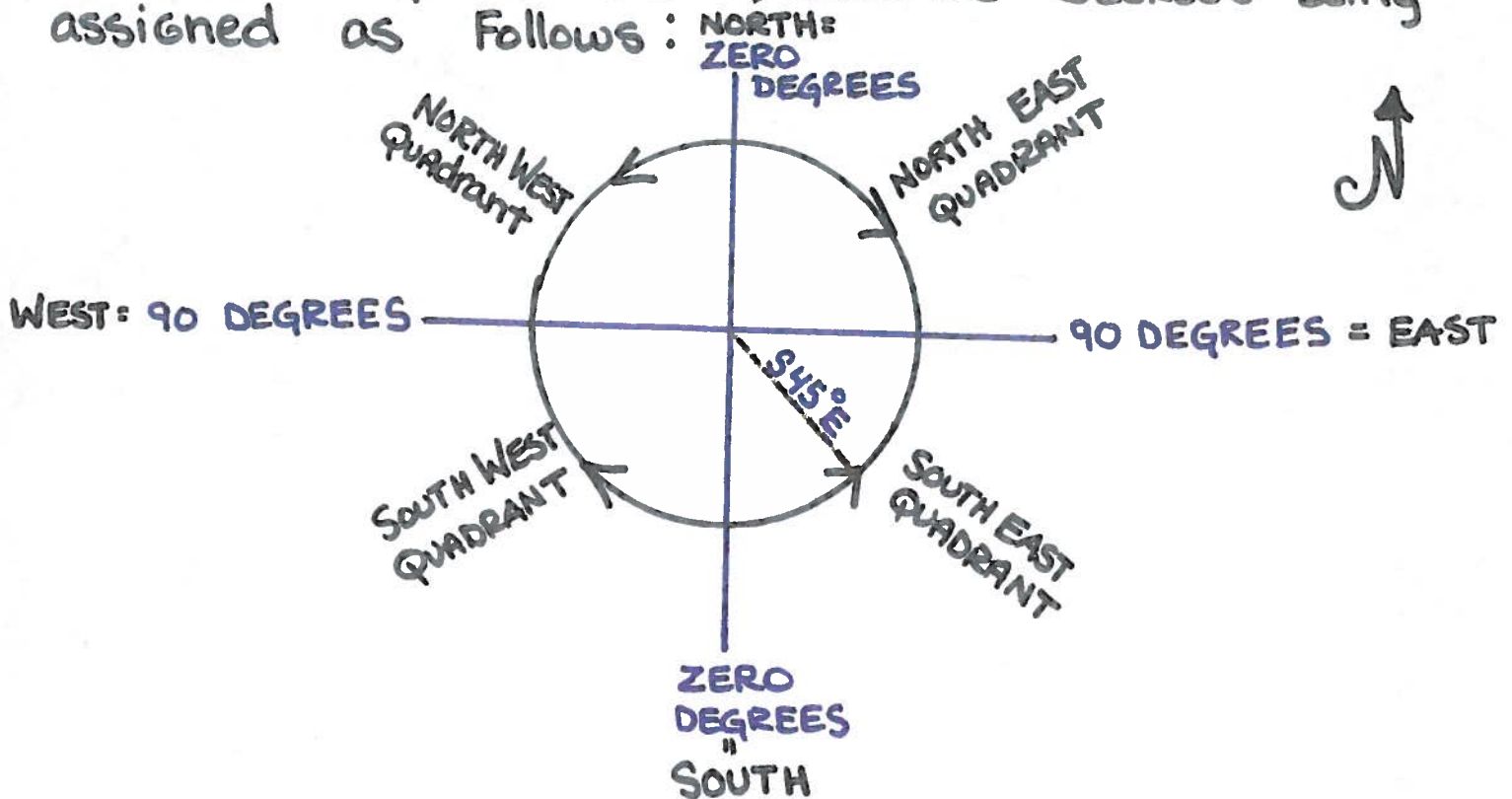


**\* NOTE \***  
SEE FORM 9600-14 for further breakdown in Subdivision corner NAMES.



## BEARINGS & AZIMUTHS

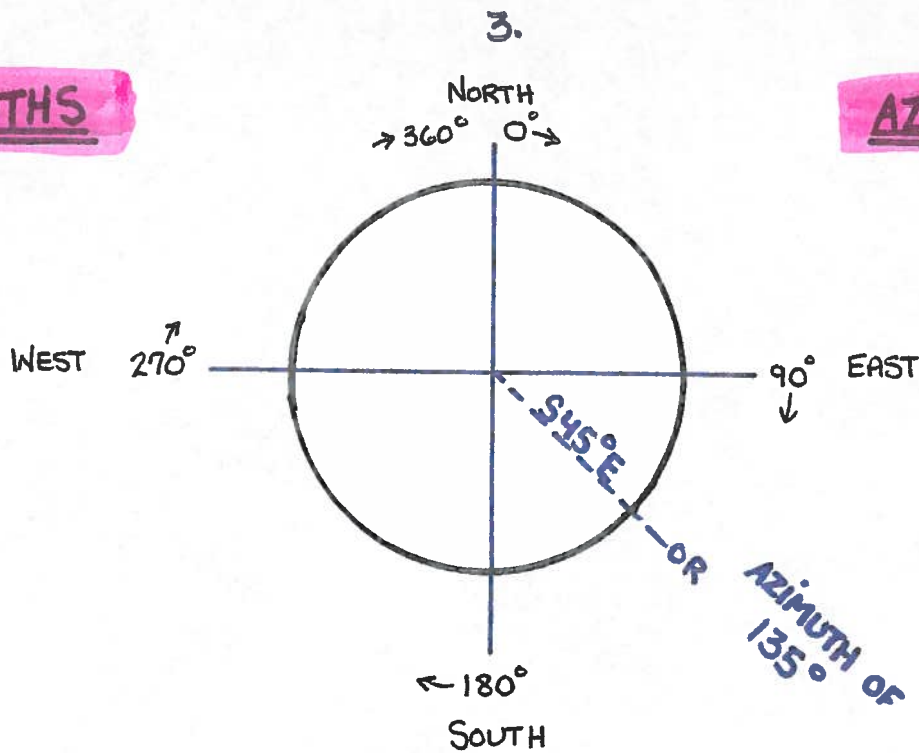
A **BEARING** is a DIRECTION, composed of DEGREES, MINUTES, and seconds. There are 4 QUADRANTS that a bearing may fall within, with the DEGREES being assigned as follows:



So - there are 90 Degrees in each Quadrant. A bearing of  $S 45^{\circ} E$  will fall exactly in the middle of EAST + SOUTH. Each DEGREE has 60 minutes, and each MINUTE has 60 seconds; much like a clock with HOURS, MINUTES + SECONDS. DUE EAST could be written:  
 N 89-59-60 E

**Azimuths** begin with Zero degrees on North, as above, but continue clockwise until ending with  $360^{\circ}$  on NORTH AGAIN.



AZIMUTHSAZIMUTHS

LIKE BEARINGS, Azimuths consist of DEGREES, MINUTES, and SECONDS.

TO GO FROM A BEARING → TO AN AZIMUTH

FOR A NE BEARING	DO NOTHING	= AZIMUTH
FOR A SE BEARING	$180^\circ - \text{BEARING}$	= AZIMUTH
FOR A SW BEARING	$180^\circ + \text{BEARING}$	= AZIMUTH
FOR A NW BEARING	$360^\circ - \text{BEARING}$	= AZIMUTH

TO GO FROM AN AZIMUTH → TO A BEARING

FOR AN AZIMUTH BETWEEN $0^\circ + 90^\circ$	DO NOTHING	= NE BEARING
FOR AN AZIMUTH BETWEEN $90^\circ + 180^\circ$	$180^\circ - \text{AZIMUTH}$	= SE BEARING
FOR AN AZIMUTH BETWEEN $180^\circ + 270^\circ$	$\text{AZIMUTH} - 180^\circ$	= SW BEARING
FOR AN AZIMUTH BETWEEN $270^\circ + 360^\circ$	$360^\circ - \text{AZIMUTH}$	= NW BEARING

\* PRACTICE TO FOLLOW .... IT IS FIRST NECESSARY TO KNOW HOW TO **ADD** AND **SUBTRACT** ITEMS THAT HAVE DEGREES, MINUTES, AND SECONDS; AS WITH AZIMUTHS, BEARINGS AND ANGLES.....



## ADDING + SUBTRACTING DEGREES, MINUTES + SECONDS

Adding or Subtracting **DEGREES** in bearings, azimuths, or angles, does not differ from traditional Addition or subtraction.... example...  $45^\circ + 10^\circ = 55^\circ$  or  $90^\circ - 30^\circ = 60^\circ$ .

HOWEVER, when minutes and seconds are involved SINCE  
 60 SECONDS = 1 MINUTE and  
 60 minutes = 1 DEGREE,  
 it is NECESSARY to think of it in terms of 60 = 1 UNIT  
 of the NEXT HIGHER VALUE.

EXAMPLE: ADD  $50^\circ 30' 45''$   
 $+ 30^\circ 40' 35''$

① ADD SECONDS FIRST; CARRY 1' TO minutes column.  
 $80'' = 1' + 20''$

② ADD MINUTES column; CARRY 1° TO DEGREES column.  
 $71' = 1^\circ + 11'$

③ ADD DEGREES column  
 $81^\circ$

SOLUTION:  $81^\circ 11' 20''$

**Subtraction** is done the same way, but it is sometimes helpful to "borrow" A DEGREE TO BEGIN with.

EXAMPLE:  
 Subtract  $180^\circ 00' 00''$   
 $- 40^\circ 15' 25''$

(EQUALS THE SAME!)  
 $\rightarrow$  CONVERT TO  $\rightarrow 179^\circ 59' 60''$   
 $- 40^\circ 15' 25''$

SOLUTION =  $139^\circ 44' 35''$

## PRACTICE

Convert BEARINGS TO AZIMUTHS & AZIMUTHS TO BEARINGS  
USING Addition + Subtraction

---

IF BEARING IS: →

1. N 15-36-45 E

2. S 80-51-20 E

3. S 60-14-34 W

4. N 73-48-50 W

THEN AZIMUTH IS:

15-36-45

99-08-40

240-14-34

286-11-10

IF AZIMUTH IS: →

1. 85-57-10

2. 146-29-37

3. 237-18-02

4. 325-47-28

THEN BEARING IS:

N 85-57-10 E

S 33-30-23 E

S 57-18-02 W

N 34-12-32 W

## HR (hours) + HMS (hours, minutes, seconds) Functions

A SPEEDIER METHOD TO +, -, x, or ÷ BEARINGS, AZIMUTHS, or angles; is TO USE THE HR (hours) Function. This function takes °, ', " and puts it in terms of a decimal. HMS takes something in decimal form back into DEGREES°, MINUTES' + SECONDS".

HR + HMS functions may be ASSIGNED TO ANY KEY. Typically, in Az., they have been AS FOLLOWS:

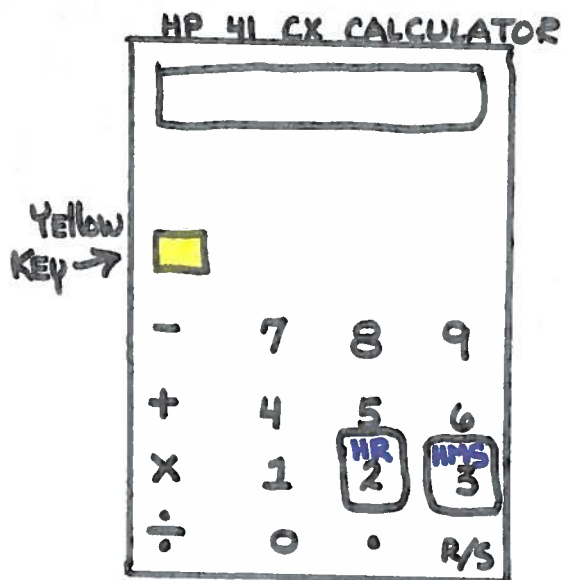
THE YELLOW KEY, called THE SHIFT key, LET'S YOU ACCESS the functions ABOVE EACH KEY.

FOR THIS EXAMPLE :

HR found @ Shift 2

HMS found @ Shift 3

$$\begin{array}{r} \text{ADD: } 16^{\circ} 45' 35'' \\ + 30^{\circ} 25' 50'' \\ \hline 47^{\circ} 11' 25'' \end{array}$$



### KEY STROKES :

### DISPLAY

- |                   |                                  |         |
|-------------------|----------------------------------|---------|
| 1. Key in 16.4535 | > Puts °, ', " into HR           | 16.7597 |
| 2. Shift 2        |                                  |         |
| 3. Key in 30.2550 | > Puts °, ', " into HR           | 30.4306 |
| 4. Shift 2        |                                  |         |
| 5. +              | - ADD THE 2 NUMBERS IN HR        | 47.1903 |
| 6. Shift 3        | - Puts the sum back into °, ', " | 47.1125 |

**SOLUTION: 47° 11' 25"**



## PRACTICE

USE HR + HMS TO +, -, x, ÷

### PROBLEM

### SOLUTION

- |    |           |   |           |   |           |
|----|-----------|---|-----------|---|-----------|
| 1. | 71-18-50  | + | 83-02-40  | = | 154-21-30 |
| 2. | 101-23-16 | + | 2-59-31   | = | 104-22-47 |
| 3. | 68-45-53  | - | 12-40-29  | = | 56-05-24  |
| 4. | 350-12-56 | - | 329-49-10 | = | 20-23-46  |
| 5. | 336-25-15 | ÷ | 4         | = | 84-06-19  |
| 6. | 25-23-40  | ÷ | 8-01-20   | = | 3-09-56   |
| 7. | 37-49-12  | x | 2         | = | 75-38-24  |
| 8. | 0-19-39   | x | 12-56-20  | = | 4-14-15   |

NOTE:

I interchangeably USE DASHES INSTEAD of the °; ' ; and " symbols.



## HMS+ AND HMS- Functions

OK - SO FAR WE'VE LEARNED 2 METHODS TO ADD OR SUBTRACT, BEARINGS, AZIMUTHS, and angles.

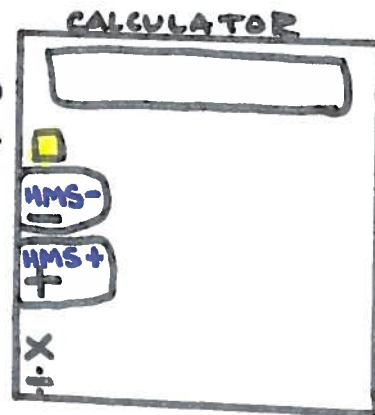
1. Do it manually.
2. USE the HR and HMS functions.

HERE'S the final, and FASTEST METHOD - USED FOR ADDING OR SUBTRACTING a BEARING, AZIMUTH, or angle ALREADY in the DEGREES, MINUTES, SECONDS" format (HMS).  
 HMS+ ADDS TWO ITEMS.  
 HMS- SUBTRACTS TWO ITEMS.

Again HMS+ and HMS- functions may be ASSIGNED TO VARIOUS KEYS.

FOR this EXAMPLE:  
 HMS+ found @ Shift +  
 HMS- found @ Shift -

Yellow KEY →



$$\begin{array}{r} \text{ADD: } 16^{\circ} 45' 35'' \\ + 30^{\circ} 25' 50'' \\ \hline \end{array}$$

### KEY STROKES

- |                              | <u>DISPLAY</u> |
|------------------------------|----------------|
| 1. Key in 16.4535            |                |
| 2. < ENTER > (hit ENTER KEY) | 16.4535        |
| 3. Key in 30.2550            | 30.2550        |
| 4. Shift +                   | 47.1125        |

**SOLUTION 47° 11' 25"**

→ \* PRACTICE with # 1-4 Previous PAGE. ←

### Program HP33s for HMS+ and HMS-

For example, I will use "P" for hms+ and "O" which shows the +/- on it for HMS-. I wrote these for the greatest economy of program steps.

To program them in you go into program mode

I will use [grn] to designate the greenish left shift key and [purple] for the purplish right shift key. I will then show the face of the key and then the function, as in [R/S:pgrm] meaning pgrm is the grn shifted R/S key.

program mode is then

[grn][R/S:pgrm]

If no other programs are in the calculator you can just start entering, otherwise you might have to step down through the existing using the rocker key in the center top of the calc to scroll through the program.

You might have to copy this into notepad or something using a courier or other fixed width font to be readable.

[grn][+ :LBL][+/-:o]	should show up as	O0001	LBL O
(Note: first character is the letter O, rest is the 4 digit program step			
[+/-]		O0002	[+/-]
[grn][+ :LBL][E:p]		P0001	LBL P
[grn][5:->HR]		P0002	->HR
[x<>y]		P0003	x<>y
[grn][5:->HR]		P0004	->HR
[+]		P0005	+
[purple][5:->HMS]		P0006	->HMS
[purple][+ :RTN]		P0007	RTN

toggle out of program mode with

[grn][R/S:pgrm]

The numbers to add or subtract have to be in the stack just like normal addition or subtraction.

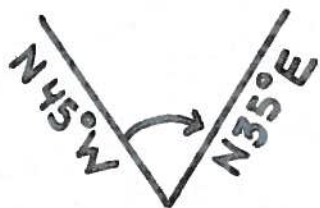
to run hms+ do [XEQ][E:p]  
to run hms- do [XEQ][+/:o]

you are responsible for setting the number of decimal places you want ahead of time using the display key up top right, or you could add a FLX 4 to the program.

If you want other letters to be used just substitute them when keying in the program.

# ANGLES

An **ANGLE** is the difference between 2 bearings OR AZIMUTHS.

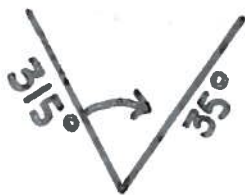


This depicts an angle of 80°.

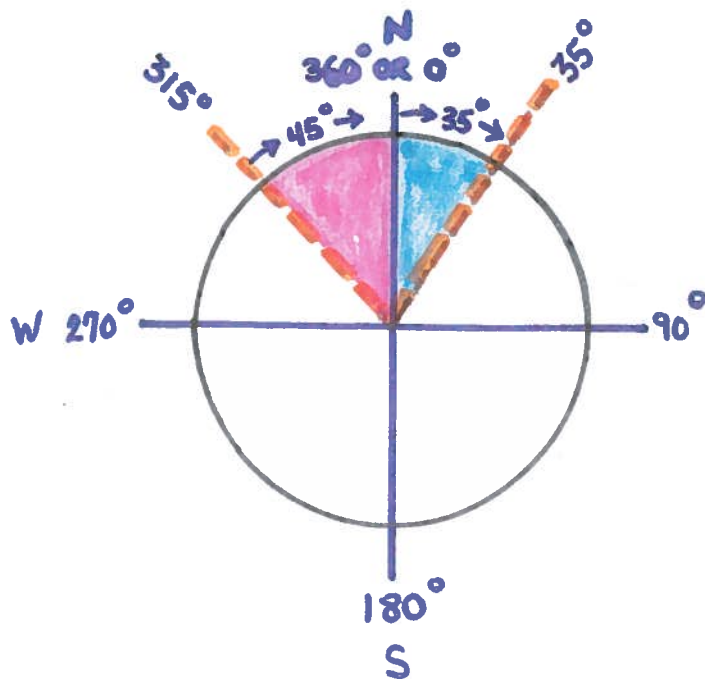
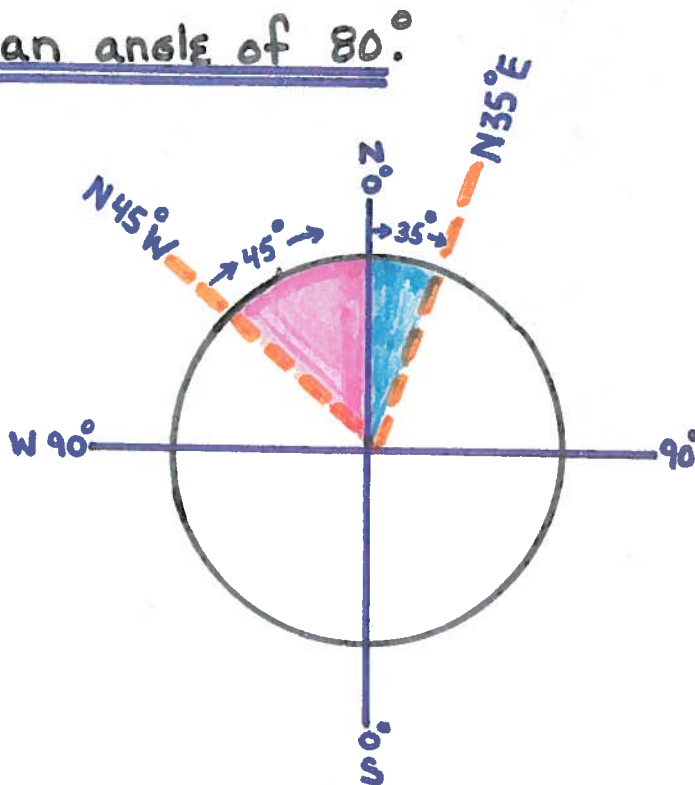
When I'm working in BEARINGS, I look at it as ADDING or SUBTRACTING KNOWN PARTS OF THE PIE.

In this CASE  $45^\circ + 35^\circ = 80^\circ$

It's the same idea for **AZIMUTHS.**



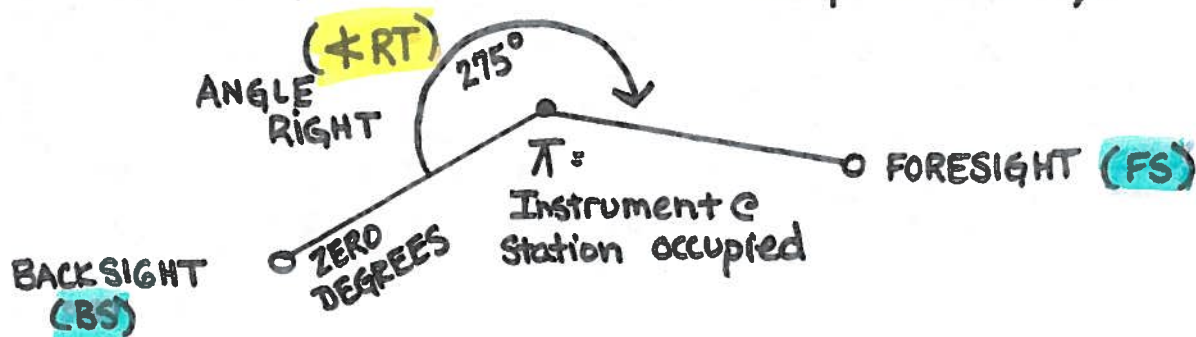
$360^\circ - 315^\circ = 45^\circ + 35^\circ = 80^\circ$





## FIELD EXAMPLES OF ANGLES

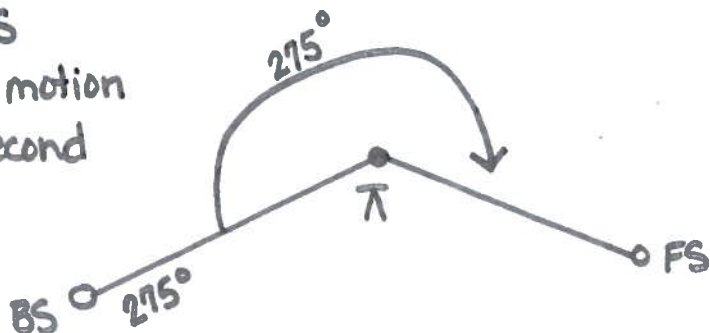
IN A typical field situation, ZERO degrees is put on the **BACKSIGHT** (usually the point LAST occupied), and an ANGLE to the RIGHT turned to the **FORESIGHT** (usually the point that will be occupied next).



The **first  $\nabla$ RT** is  $275^\circ$ . To check human error as well as any error in the alignment of the instrument scope (Collimation), a second angle is turned with the Scope "FLOPPED"  <sup>$180^\circ$</sup>  or in the opposite direction that it was the first time.

The **Second  $\nabla$ RT** is ADDED to the first by HOLDING the  $275^\circ$  until it is placed on the backsight (JUST AS ZERO WAS originally), and then releasing the motion to allow  $\nabla 2$  to be Accumulated.

1. Lock  $275^\circ$  in GUN
2. Sight BS
3. RELEASE motion  
To Add second  
 $\nabla$ .



## FINDING THE MEAN OF 2 $\angle$ RIGHTS

To determine if the  $\angle$  Rt. turned is GOOD or NOT, the second  $\angle$  Rt is divided by two to get the **MEAN  $\angle$  Rt.** If the MEAN is within 5 seconds of the FIRST  $\angle$  Rt. the ANGLES ARE generally considered to be good, depending on the instrument.

EXAMPLE:  $\angle$  Rt

1) 20-15-28	>>>	3 seconds = OK ANGLES.
2) 40-30-50		
3) 20-15-25		

**NOTE:** An instrument will never read more than  $360^\circ$ , so if the first  $\angle$  is greater than  $180^\circ$ , you MUST ADD  $360^\circ$  TO  $\angle 2$  before dividing by 2.

EXAMPLE:  $\angle$  Rt

1) 275-00-00	$\rightarrow$ Greater than $180^\circ$
2) 190-00-00	$\rightarrow (190+360) \div 2$
3) 275-00-00	= FLAT ANGLE, OK.

PRACTICE: FIND THE MEAN  $\angle$  RT .....  $\hat{=}$  is it OK?

#1	$\angle$ Rt 1) 30-19-50	2) 60-39-40	m)
#2	$\angle$ Rt 1) 145-18-22	2) 290-36-50	m)
#3	$\angle$ Rt 1) 76-39-48	2) 153-19-25	m)
#4	$\angle$ Rt 1) 122-31-39	2) 245-03-00	m)
#5	$\angle$ Rt 1) 195-20-20	2) 30-40-50	m)
#6	$\angle$ Rt 1) 8-40-30	2) 17-21-20	m)
#7	$\angle$ Rt 1) 275-06-12	2) 190-12-37	m)
#8	$\angle$ Rt 1) 325-46-01	2) 291-32-02	m)
#9	$\angle$ Rt 1) 181-58-45	2) 3-57-42	m)
#10	$\angle$ Rt 1) 139-27-19	2) 278-54-45	m)

SOLUTIONSMEAN ANGLE RIGHT

	$\neq 2$	MEAN		$\neq 1$	OK?
# 1	$60-39-40 \div 2 =$	<u>30-19-50</u>	COMPARE TO	30-19-50	<u>OK</u> FLAT
# 2	$290-36-50 \div 2 =$	145-18-25	"	145-18-22	OK 3"
# 3	$153-19-25 \div 2 =$	76-39-43	"	76-39-48	OK 5"
# 4	$245-03-00 \div 2 =$	122-31-30	"	122-31-39	NOT OK 9"
# 5	$[(360+(30-40-50)) \div 2 =$	195-20-25	"	195-20-20	OK 5"
# 6	$17-21-20 \div 2 =$	8-40-40	"	8-40-30	NOT OK 10"
# 7	$[(190-12-37)+360] \div 2 =$	275-06-19	"	275-06-12	NOT OK 7"
# 8	$[(360+(291-32-02))] \div 2 =$	325-46-01	"	325-46-01	OK FLAT
# 9	$[(3-57-42)+360] \div 2 =$	181-58-51	"	181-58-45	NOT OK 6"
# 10	$278-54-45 \div 2 =$	139-27-23	"	139-27-19	OK 4"

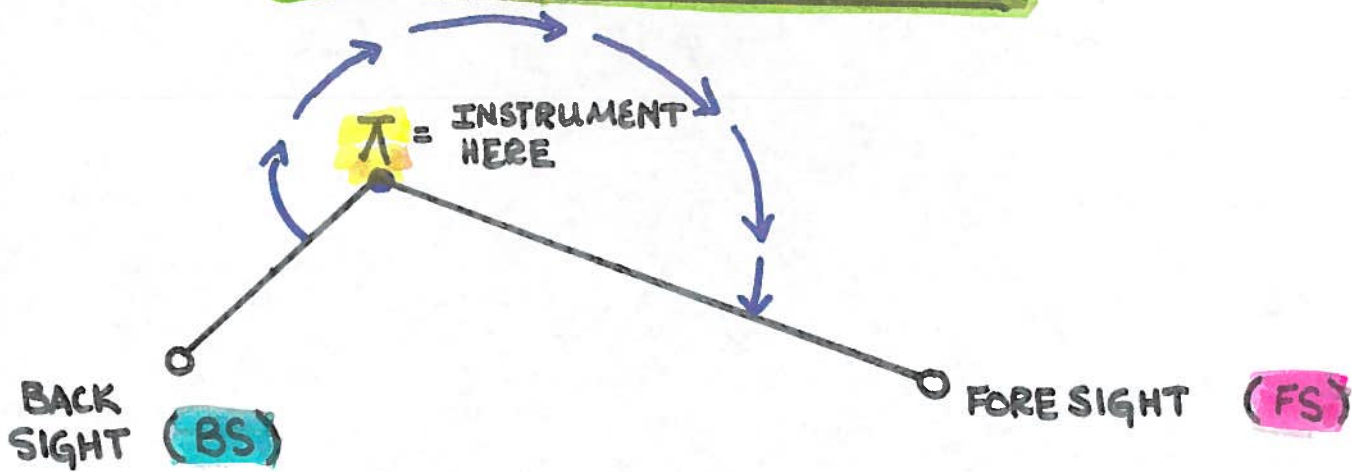
\*(HP41!)

KEY STROKES TO SOLVE #5

	DISPLAY SHOWS
1. Key in $\neq 2$	30-40-50
2. <ENTER>	30-40-50
3. (* Since $\neq 1$ is greater than $180^\circ$ ; ADD $360^\circ$ ) Key in 360	360
4. +	390-40-50
5. HR (shift 2)	390-68-06
6. Key in 2	2
7. $\div$	195-34-03
8. HMS (shift 3)	<u>195-20-25</u>



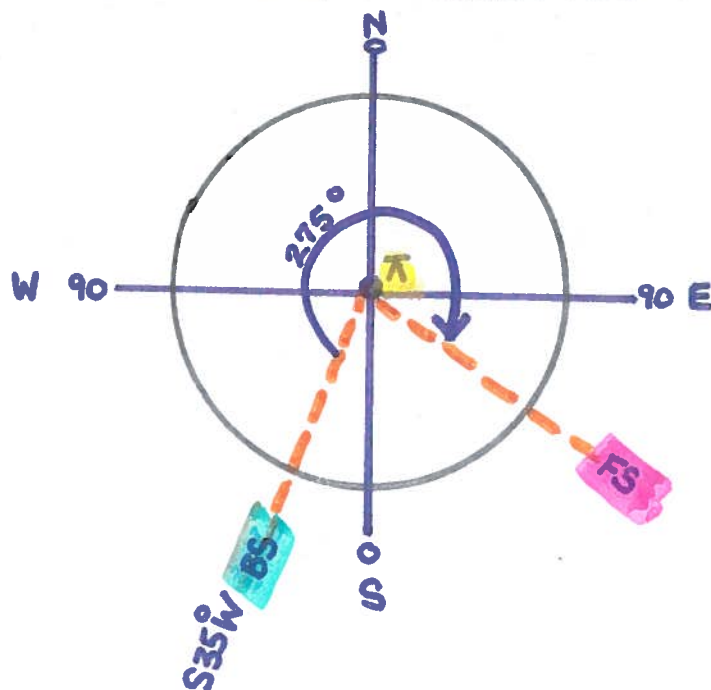
## MORE FIELD EXAMPLES OF ANGLES

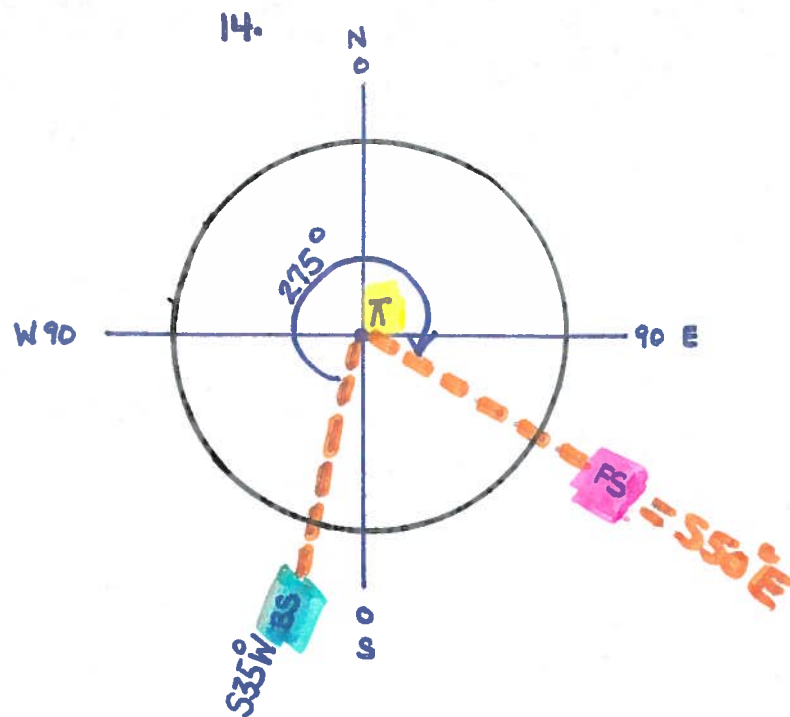


We typically turn our ANGLES from the backsight, TO THE RIGHT, TO OUR FORESIGHT.  
 \* (Most calculator + computer programs ARE set up for using + RT.)

Problem:

If the Backsight bearing is  $S35^{\circ}W$  and the Angle Right is  $275^{\circ}$  (+ RT).  
 WHAT IS THE FORESIGHT BEARING??





Solution: There ARE SEVERAL WAYS TO APPROACH this problem, I will explain what WORKS BEST for me.

- #1. Change the BS bearing to an AZIMUTH.  $180^\circ + 35^\circ = 215^\circ$
- #2. Add to this the  $\nless RT$ .  $215^\circ + 275^\circ = 490^\circ$ .
- #3. Since we know that a circle can never have MORE than  $360^\circ$ ; as our example shows, merely subtract  $360^\circ$  from the total.  $490^\circ - 360^\circ = 130^\circ$
- #4. Change the AZIMUTH ANSWER back to a bearing — if desired.  $130^\circ = \text{S } 50^\circ \text{ E}$ .

## PRACTICE

BACKSIGHT BEARING       $\nless RT$       ? FORESIGHT BEARING

1.	N $10^\circ$ E	$60^\circ$
2.	S $25^\circ$ E	$175^\circ$
3.	S $89-30-45$ W	$15^\circ$
4.	N $76-48-00$ W	$95-20-35$
5.	S $35-08-16$ E	$315-18-08$
6.	S $50-50-50$ W	$76-42-31$

NOTE: IT HELPS TO DRAW A SKETCH !!!

## SOLUTIONS TO Adding an $\pm$ Rt to a BS BEARING.

#1. BS bearing =  $N10^{\circ}E$

$$\text{Azimuth} = 10^{\circ} + \pm \text{Rt } 60^{\circ} = \text{Azimuth } 70^{\circ} = \text{FS BEARING } \boxed{N70^{\circ}E.}$$

#2. BS bearing =  $S25^{\circ}E$

$$\text{Azimuth} = 155^{\circ} + \pm \text{Rt } 175^{\circ} = \text{Azimuth } 330^{\circ} = \text{FS BEARING } \boxed{N30^{\circ}W.}$$

#3. BS bearing =  $S89-30-45W$

$$\text{Azimuth} = 269-30-45 + \pm \text{Rt } 15^{\circ} = \text{Azimuth } 284-30-45 = \text{FS bearing } \boxed{N75-29-15W.}$$

#4. BS bearing =  $N70-48-00W$

$$\text{Azimuth} = 289-12-00 + \pm \text{Rt } 95-20-35 = 384-32-35$$

\* \* NOTE: \* \* \*

$$\underline{- 360-00-00}$$

$$\text{AZIMUTH} = 24-32-35 =$$

-360 !!!!

FS BEARING

$$\boxed{N24-32-35E}$$

#5. BS bearing =  $S35-08-16E$

$$\text{Azimuth} = 144-51-44 + \pm \text{Rt } 315-18-08 = 460-09-52$$

\* \* OVER 360 !!!

$$\underline{- 360-00-00}$$

$$\text{AZIMUTH} = 100-09-52 =$$

FS BEARING

$$\boxed{S79-50-08E}$$

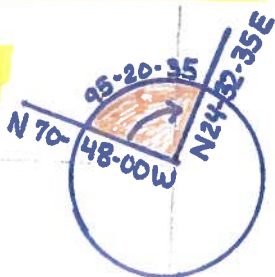
#6. BS bearing =  $S50-50-50W$

$$\text{Azimuth} = 230-50-50 + \pm \text{Rt. } 76-42-31 = \text{Azimuth } 307-33-21 = \text{FS BEARING}$$

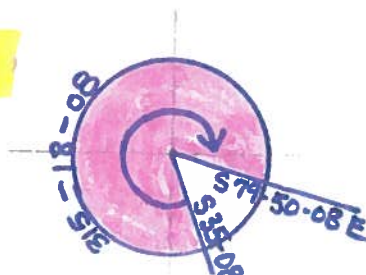
$$\boxed{N52-26-39W}$$

### SKETCH EXAMPLES:

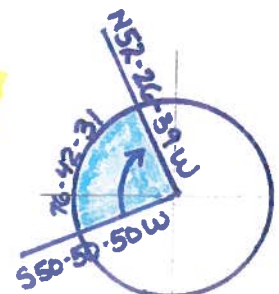
#4.



#5.



#6.





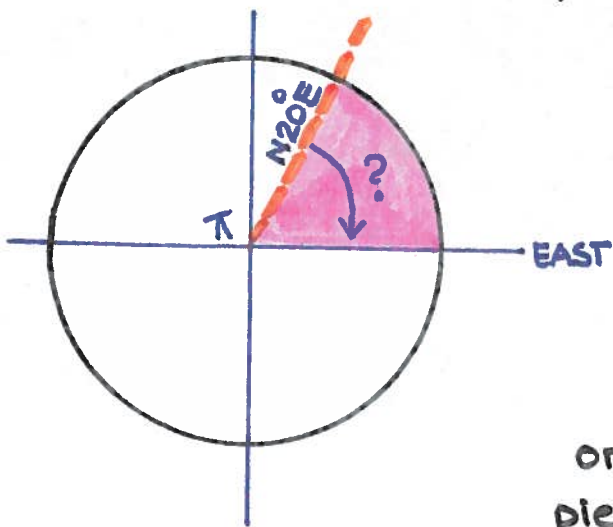
4 RT

FIELD ANGLES CONTINUED

4 RT

Following the same principles, it is often necessary to calculate the **Angle RIGHT** from a known backsight to a desired foresight.

EXAMPLE: OUR BS BEARING IS  $N 20^{\circ} E$ ; we want to go DUE EAST, What 4Rt. do we need to turn.



AGAIN, there ARE many ways to arrive at a solution, depending on how you look at it.

For 4Rt's I find it easier to work in bearings, and simply ADD or subtract whatever pieces of the pie that I need.

In this case  $\frac{1}{4}$  of the pie =  $90^{\circ} - 20^{\circ} = 4Rt\ 70^{\circ}$ .  
My best advice is to DRAW A PICTURE.

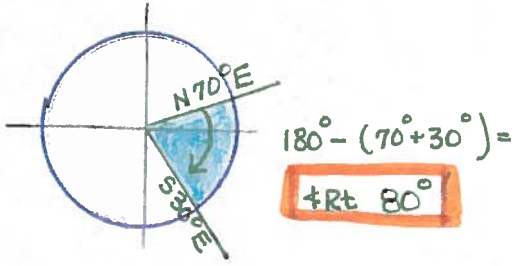
PRACTICEBS BEAR.FS BEAR.4RT?

- |     |                  |                  |  |
|-----|------------------|------------------|--|
| 1.  | $N 70^{\circ} E$ | $S 30^{\circ} E$ |  |
| 2.  | $S 75^{\circ} E$ | $S 25^{\circ} E$ |  |
| 3.  | $S 40^{\circ} W$ | $N 55^{\circ} W$ |  |
| 4.  | $N 10^{\circ} W$ | $S 40^{\circ} E$ |  |
| 5.  | $N 80^{\circ} E$ | $N 20^{\circ} W$ |  |
| 6.  | $S 5^{\circ} E$  | $N 75^{\circ} E$ |  |
| 7.  | $S 55-40-25 W$   | $S 89-59-50 W$   |  |
| 8.  | $N 35-15-48 W$   | $S 20-58-36 W$   |  |
| 9.  | $S 45-10-30 W$   | $N 65-11-21 E$   |  |
| 10. | $N 28-28-28 E$   | $N 8-08-08 E$    |  |

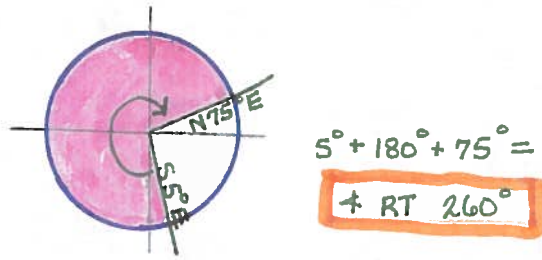
# Solutions to ✚RT PRACTICE

AGAIN - this is just how I'd look at it — find what works best for you!!

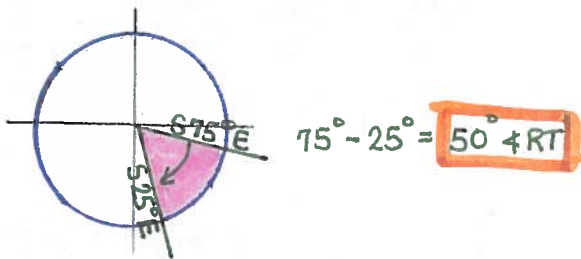
#1



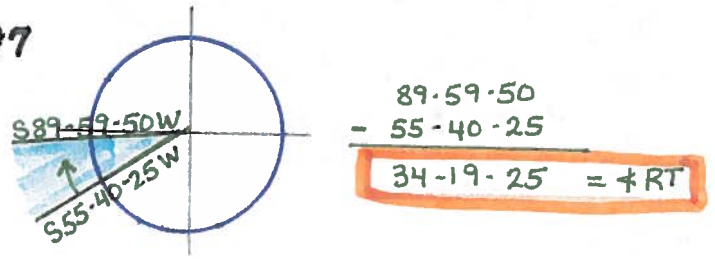
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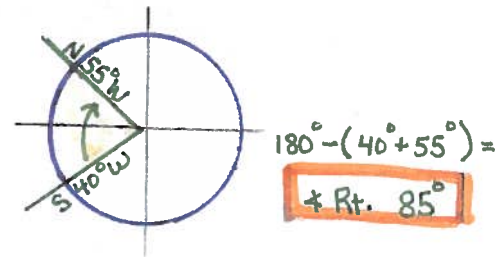
#2



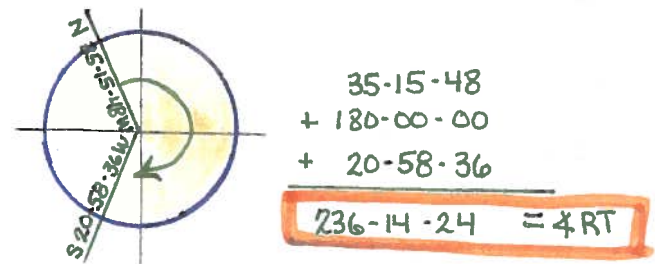
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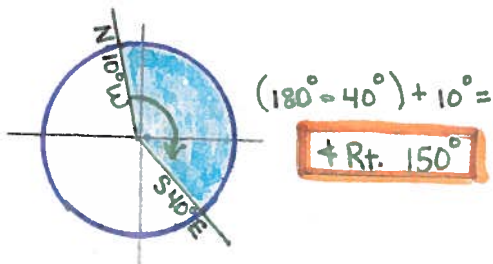
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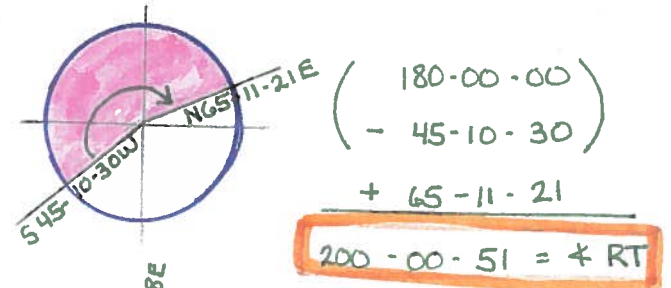
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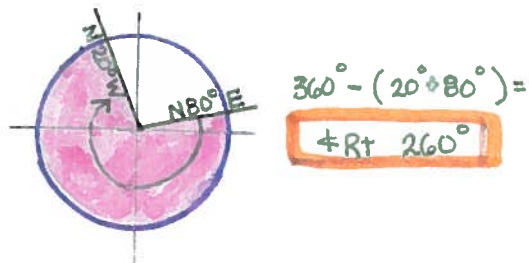
#4



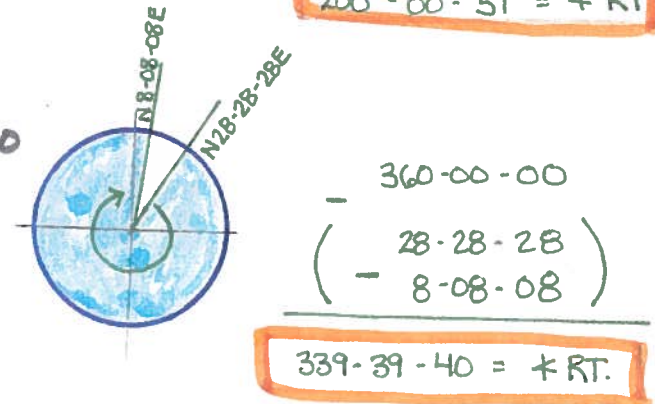
#9



#5



#10

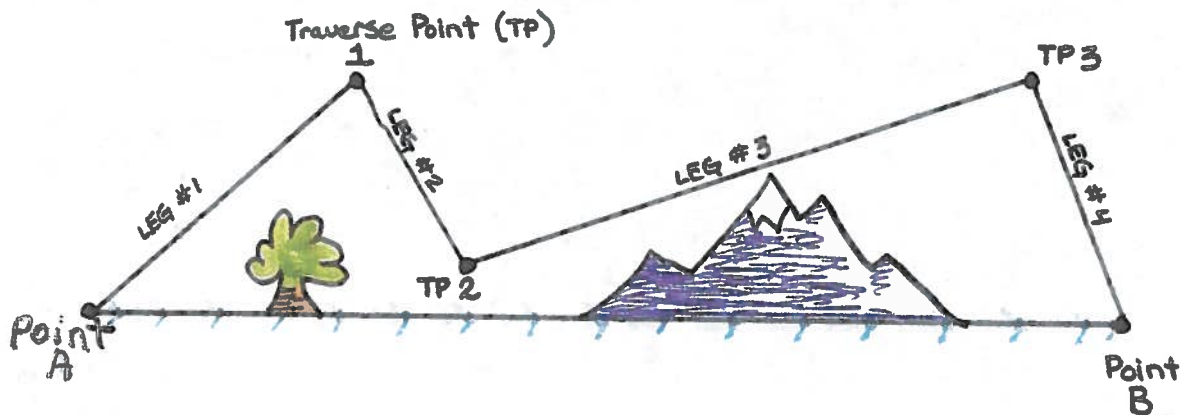


P → R

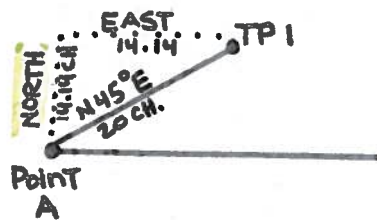
LATITUDES & DEPARTURESBEARINGS & DISTANCES

R → P

A survey traverse is the accumulation of many legs (1 bearing; 1 distance) to find the overall bearing and distance between 2 points; when a direct line of sight is not possible, or distances are too great.



If the bearing and distance for Leg #1 is  $N45^\circ E$ , for 20 chains; Then, between Point A + TP 1 we have travelled NORTH 14.14 CH. and EAST 14.14 CH.



\* EACH bearing & distance can be broken down into a LATITUDE (how far we go North or South); and a DEPARTURE (how far we go EAST or WEST).

$$\begin{array}{l} N45^\circ E \\ 20.00 \end{array} = \begin{array}{l} N 14.14 \\ E 14.14 \end{array} = \text{RECTANGULAR COORDINATES}$$

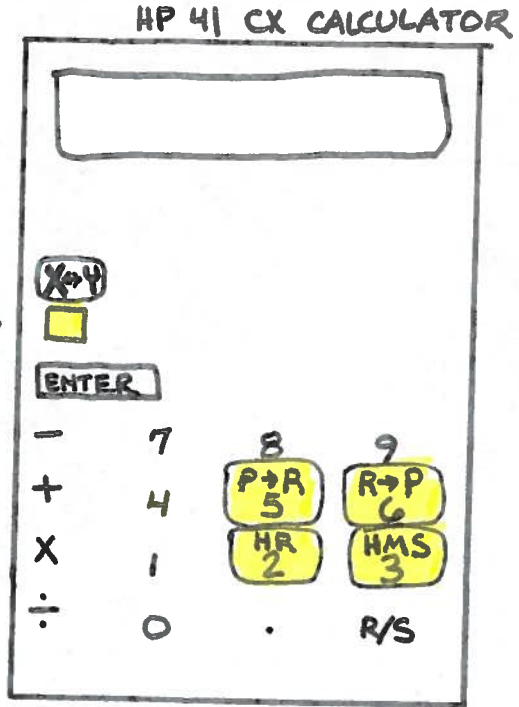
By the same token, a latitude and departure may be converted into a distance + bearing, THE POLAR COORDINATES.



19.  
R → P ⇌ P → R

The yellow key, called the SHIFT key, lets the calculator access the functions ABOVE each key.

**\* Note \***: The location of the HR, HMS functions may vary!! CHECK YOUR KEY ASSIGNMENTS → SHIFT, CATALOG, G. ← (HP41)



Yellow key →

**KEY STROKES:**

N 45.3530 E 20 CH.

Polar to Rectangular

1. Key in BEARING (HMS)
2. Shift 2 (Puts INTO HR)
3. Key in Distance
4. Shift 5 (P → R)
5. LATITUDE is Displayed.
6. X ↔ Y
7. DEPARTURE is displayed.

EXAMPLE:                      DISPLAYED:

1. Key in: 45.3530
2. Shift 2 45.5917
3. Key in: 20.00
4. Shift 5
5. LATITUDE = 13.9953
6. X ↔ Y
7. Departure = 14.2874

ANSWER: N 13.9953

N 13.9953 E 14.2874

Rectangular to Polar

1. Key in DEPARTURE
2. ENTER
3. Key in Latitude
4. Shift 6 (R → P)
5. DISTANCE is displayed.
6. X ↔ Y
7. BEARING is displayed (in HR)
8. Shift 3 (Puts bearing into HMS)

EXAMPLE:                      DISPLAYED:

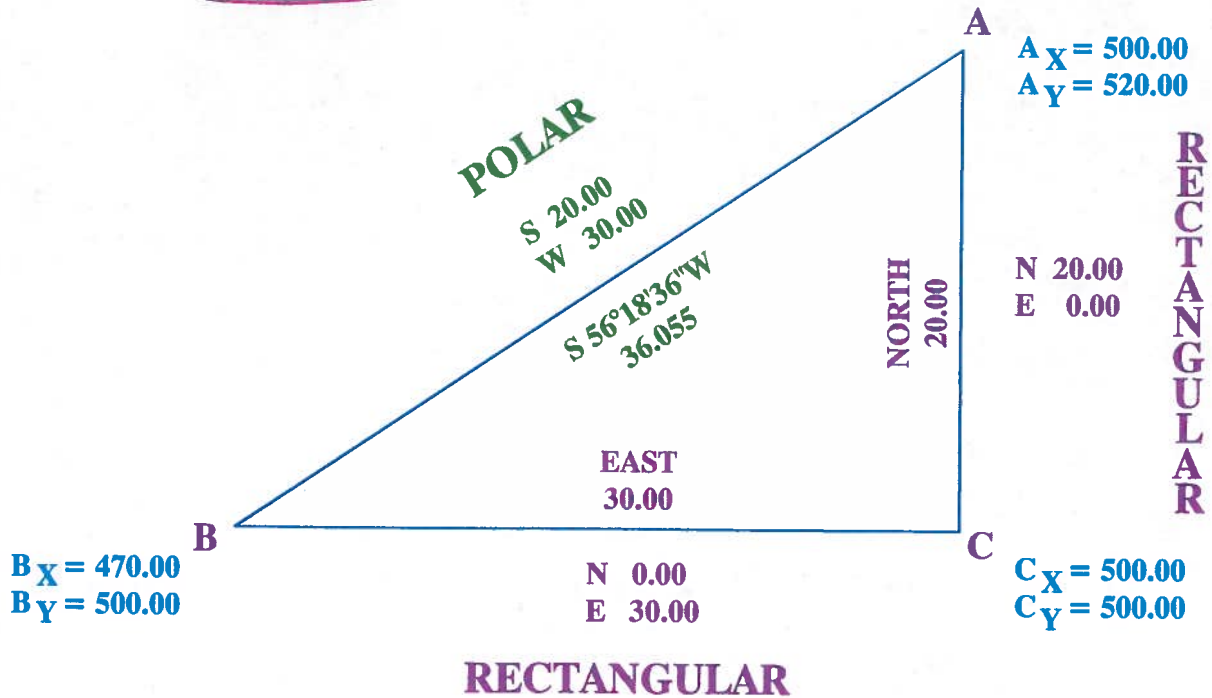
1. Key in: 14.2874
2. ENTER
3. Key in: 13.9953
4. Shift 6
5. DISTANCE = 20.0000
6. X ↔ Y
7. Bearing = 45.5918 (in HR)
8. Shift 3 = 45.3530

ANSWER:

# Polar and Rectangular Calculator Functions



\*Example using the  
hp 33s



## KEYSTROKES

### **R to P**

1. Key in Departure
2. <Enter>
3. Key in Latitude
4. "Green Shift" + **[4]**
5. Distance is displayed
6. X ↔ Y
7. Bearing is displayed
8. "Purple Shift" + **[5]** = HMS

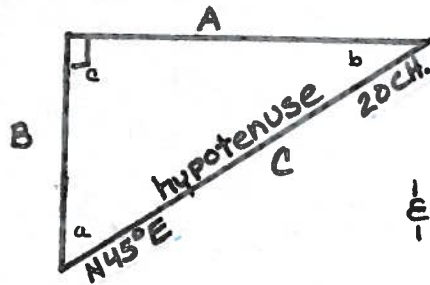
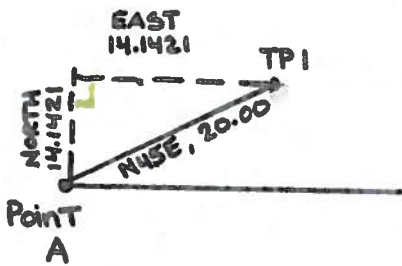
### **P to R**

1. Key in bearing
2. "Green Shift" + **[5]** = HR
3. Key in distance
4. "Purple Shift" + **[4]**
5. Latitude is displayed
6. X ↔ Y
7. Departure is displayed.

# R → P & P → R

12-90  
8-06

HERES the basic mathematical basis. THE SURVEY leg can be seen as a right triangle, and defined by the Pythagorean Theorem:  $A^2 + B^2 = C^2$  } Solves DISTANCE.

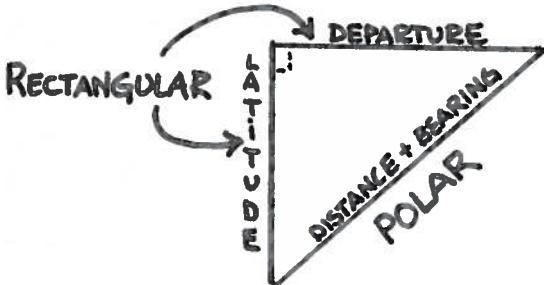


THEN :

$$\begin{aligned} \tan b &= \frac{B}{A} \\ \text{OR} \\ \tan a &= \frac{A}{B} \end{aligned} \quad \left. \vphantom{\begin{aligned} \tan b &= \frac{B}{A} \\ \text{OR} \\ \tan a &= \frac{A}{B} \end{aligned}} \right\} \text{SOLVES } \angle \text{'s}$$

ie  $14.1421^2 + 14.1421^2 = 20^2$

\* To go from a bearing & distance to a latitude & departure (AND), QUICKLY → (See Box for HP33s) → → HP41 calculators have a P → R, R → P function, or POLAR → Rectangular.



← + →  $\theta, r$  = Lat. & Dep. TO Br. & Dist.  
 ↶ + →  $y, x$  = Br. & Dist. TO Lat. & Dep.

\* Key STROKES

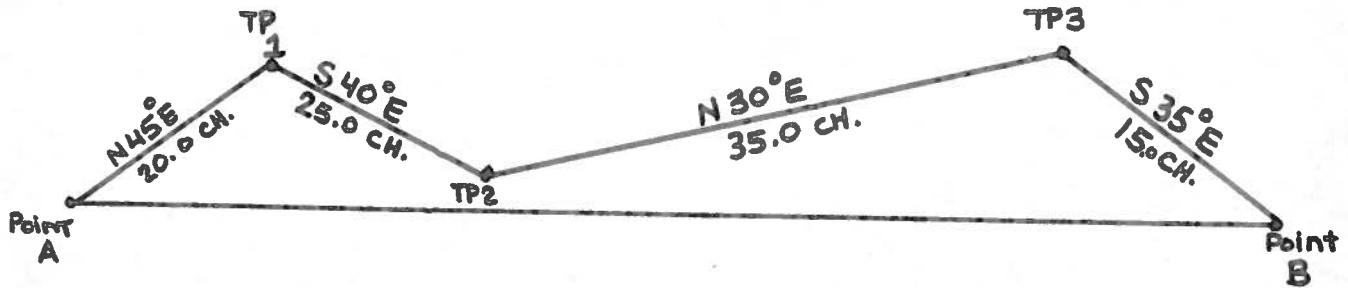
\* **P → R** - takes a bearing + distance and converts to a latitude + departure.

\* **R → P** - takes a latitude + departure and converts to a bearing + distance.

- R → P**
1. Key in DEPARTURE
  2. Enter
  3. Key in LATITUDE
  4. 'Green Shift' + 4
  5. Distance is displayed.
  6.  $X \leftrightarrow Y$
  7. Bearing is displayed (in HR)
  8. 'Purple Shift' + 5 = HMS.

- P → R**
1. Key in Bearing (HMS)
  2. 'Green Shift' + 5 = (HR)
  3. Key in Distance
  4. 'Green Shift' + 4
  5. Latitude is displayed
  6.  $X \leftrightarrow Y$
  7. Departure is displayed.



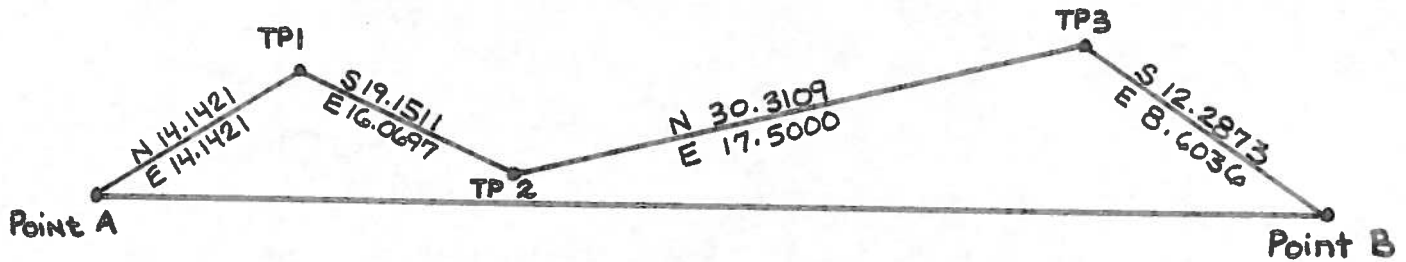


GIVEN the above distances & bearings; Calculate the latitude + departure for each leg of the traverse.

BEARING	Distance	Latitude	Departure
1. N 45° E	20.00	N 14.1421	E 14.1421
2. S 40° E	25.00	S 19.1511	E 16.0697
3. N 30° E	35.00	N 30.3109	E 17.5000
4. S 35° E	15.00	S 12.2873	E 8.6036

#### MORE PRACTICE

5. S 89-59-30 W	16.54	S 0.0024	W 16.5400
6. N 73-21-20 W	12.73	N 3.6463	W 12.1966
7. S 60-18-47 E	58.22	S 28.8341	E 50.5783
8. N 0-02-18 E	3.92	N 3.9200	E 0.0026
9. S 5-59-40 W	79.50	S 79.0653	W 8.3023
10. N 44-20-30 W	18.31	N 13.0950	W 12.7975
11. N 38-01-59 E	40.05	N 31.5456	E 24.6754
12. S 16-49-27 W	00.70	S 0.6700	W 0.2026
13. S 88-08-10 E	20.89	S 0.6795	E 20.8789
14. N 70-22-48 W	45.66	N 15.3317	W 43.0090
15. S 3-15-07 W	5.99	S 5.9804	W 0.3398



Given the above latitudes & departures, Calculate the distance and bearing for each leg of the traverse.

Latitude	Departure	Distance	Bearing
----------	-----------	----------	---------

1. N 14.1421	E 14.1421	20.0	N 45° E
2. S 19.1511	E 16.0697	25.0	S 40° E
3. N 30.3109	E 17.5000	35.0	N 30° E
4. S 12.2873	E 8.6036	15.0	S 35° E

### MORE PRACTICE

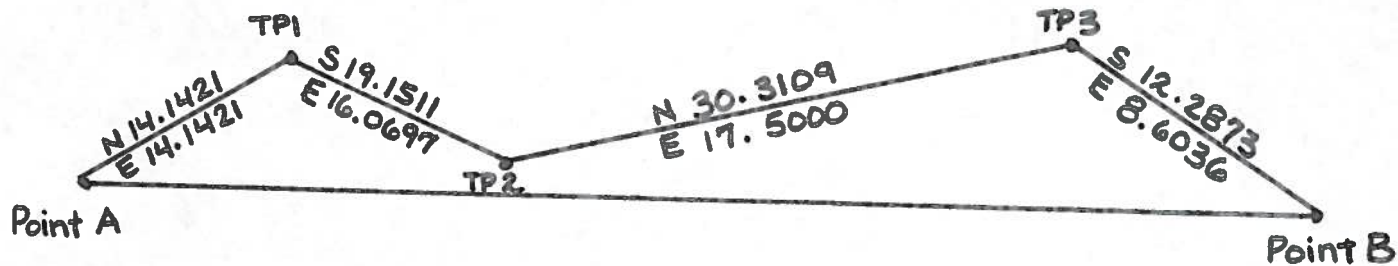
5. S 80.0000	W 0.1000	80.0001	S 0-04-18 W
6. N 13.0000	W 45.0000	46.8402	N 73-53-12 W
7. N 26.0000	E 51.0000	57.2451	N 62-59-14 E
8. S 8.0000	E 70.0000	70.4557	S 83-28-49 E
9. S 39.8888	W 0.5000	39.8919	S 0-43-05 W
10. N 2.1555	W 20.0000	20.1158	N 83-50-55 W
11. N 100.0000	E 100.0000	141.4214	N 45 E
12. S 35.0000	E 10.0000	36.4005	S 15-56-43 E
13. S 79.3888	W 1.2222	79.3982	S 0-52-55 W
14. N 16.1786	W 57.0032	59.2546	N 74-09-18 W
15. N 0.0123	E 0.8546	0.8547	N 89-10-31 E

TOTALS

# Totals

12-90

TOTALS



\* To find the **TOTAL** ( $\Sigma$ ) latitude + departure between Point A and Point B, we simply ADD ALL the legs together.

→ ASSIGN a positive **+** Value to all **Northings** + **EASTINGS**.

→ ASSIGN a negative **-** Value to all **Southings** + **Westings**.

Summation ( $\Sigma$ ) of Latitudes

1. + 14.1421
2. - 19.1511
3. + 30.3109
4. - 12.2873

TOTAL

LAT. + 13.0146  
= N 13.0146

Summation ( $\Sigma$ ) of Departures

1. + 14.1421
2. + 16.0697
3. + 17.5000
4. + 8.6036

TOTAL

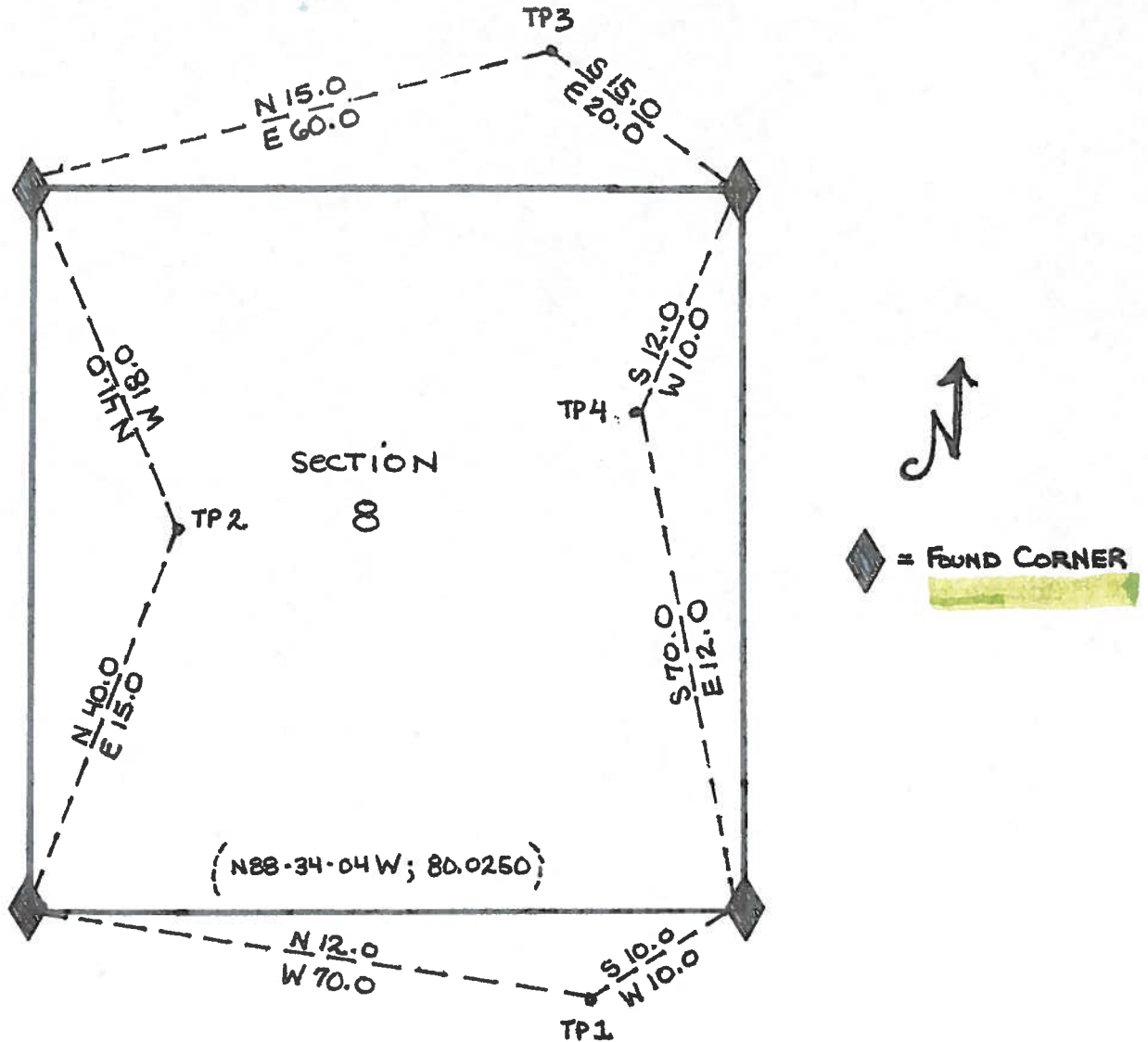
DEP. + 56.3154  
= E 56.3154

★ N 13.0146  
E 56.3154 > TOTAL LAT. + DEP. BETWEEN point A + point B.

★ N 76.59.14 E  
57.7997 > TOTAL distance + bearing between point A + point B.



## Sample Traverse



Here's a sample traverse around sec. 8. Begin at the SE cor., traverse clockwise around the section. Compute the total distance and bearing between found corners on the perimeter of sec. 8.

South Bdy.	$-10 + 12 = +2.0$	$\Sigma$ LAT. = N 2.00	=	N 88-34-04 W
	$-10 + (-70) = -80.0$	$\Sigma$ DEP. = W 80.00		80.0250
WEST Bdy.	$+40 + 41 = +81.0$	= N 81.0	=	N 2-07-16 W
	$+15 + (-18) = -3.0$	= W 3.0		81.0555

NORTH Bdy.

$$+15.0 + (-15.0) = 0.00$$

$$= N-S 0.00$$

= EAST

$$+60.0 + 20.0 = 80.0$$

$$= E 80.00$$

80.00

EAST Bdy.

$$-12.0 + (-70.0) = -82.00$$

$$= S 82.00$$

= S 1-23-50 E

$$-10.0 + 12.0 = +2.00$$

$$= E 2.00$$

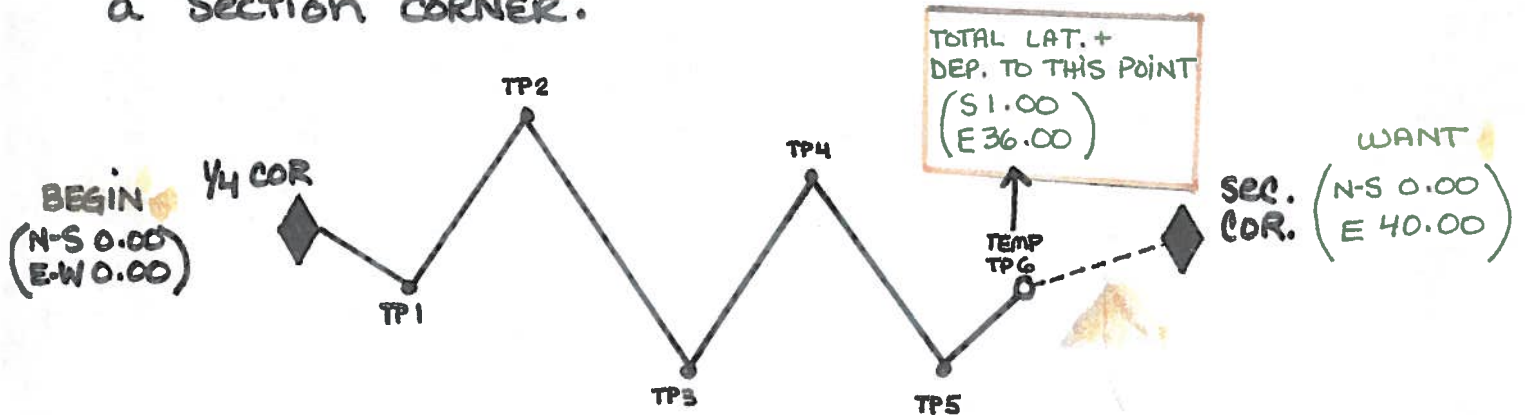
82.0244

INVERSING

OR DETERMINING THE BEARING & DISTANCE BETWEEN TWO SETS OF RECTANGULAR COORDINATES.

The procedure for getting from where you ARE to where you want to GO is called INVERSING.

FOR instance, we want to traverse from a found  $\frac{1}{4}$  CORNER, DUE EAST for 40 chains, to find a SECTION CORNER.



Beginning with zero, and traversing to Temp TP6, the total Latitude + Departure to this point is S 1.00 and E 36.00.

To find how far we need to go to the section corner, it's just a matter of adding or subtracting LATS. + DEPS.

ARE @ TP6

S 1.00  
E 36.00

WANT for sec. COR.

N-S 0.00  
E 40.00

GO

N 1.00 OR N 75-57-50E  
E 4.00 4.12 CH.

A CORNER move is nothing more than inversing from a Temp, to the calculated position for A CORNER.



## INVERSING (Cont.)

Hints for solving practice problems: 1. Work in LATS. + DEPS. 2. Pay attention to Directions; as in Adding E's to W's OR N's to S's. 3. DRAW A Picture - see the relation of where you are To where you want to go.

### PRACTICE

<u>You ARE</u>	* Go ahead and round lats. + DEPS. To two decimal places *	<u>You WANT</u>	<u>GO</u>	<u>BEARING + DISTANCE</u>
#1) N 16.00 E 50.00		57.87 CH N 71-52-41 E		
#2) S 10.00 W 15.00		S 59-02-10 W 23.32		
#3) S 5.00 E 45.00		N 86-25-25 E 80.16		
#4) N 8.00 E 1.00		S 87-03-52 W 39.05		
#5) S 5.00 E 4.00		NORTH 40.00		
#6) N 39.00 W 5.00		N 0-42-58 W 80.01		
#7) N 45.00 W 5.00		N 14-02-10 E 41.23		

## Solutions to Inversing Problems

ARE	WANT	GO	BR.+ DIST.
#1) N 16.00 E 50.00	N 18.00 E 55.00	N 2.00 E 5.00	N 68-11-55 E 5.39
#2) S 10.00 W 15.00	S 12.00 W 20.00	S 2.00 W 5.00	S 68-11-55 W 5.39
#3) S 5.00 E 45.00	N 5.00 E 80.00	N 10.00 E 35.00	N 74-03-17 E 36.40
#4) N 8.00 E 1.00	S 2.00 W 39.00	S 10.00 W 40.00	S 75-57-50 W 41.23
#5) S 5.00 E 4.00	N 40.00 W 0.00	N 45.00 W 4.00	N 5-04-47 W 45.18
#6) N 39.00 W 5.00	N 80.00 W 1.00	N 41.00 E 4.00	N 5-34-20 E 41.19
#7) N 45.00 W 5.00	N 40.00 E 10.00	S 5.00 E 15.00	S 71-33-54 E 15.81

\* Step by Step solution to #4





# Step by Step Solution to #4 Inversing Problem



#4

ARE  
N 8.00  
E 1.00

WANT  
S 87-03-52 W  
39.05

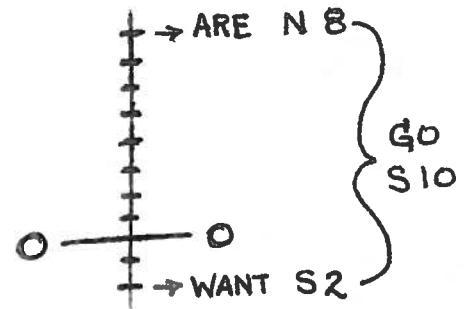
GO	BR. + DIST.
?	?

1. Convert where you **Want** to a Latitude + Departure.

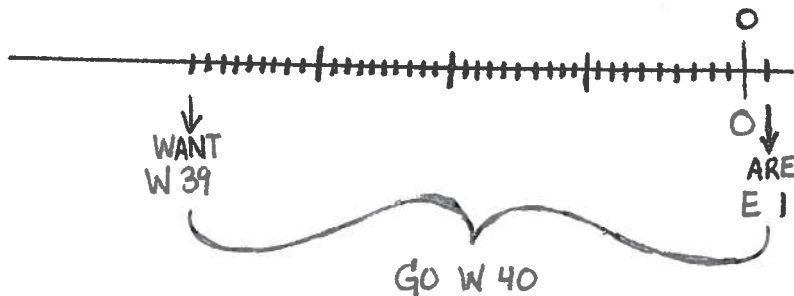
S 87-03-52 W = S 2.00  
39.05 = W 39.00

2. Compare the LATS. Draw a picture. A number line with zero in the middle of north values and south values, works well.

We ARE N 8, to get to S 2, we'd GO a total of **10 CHAINS S.**



3. Compare the DEPS. Draw a picture.



We ARE E 1, to get to W 39, we'd GO a total of

**40 chains W.**

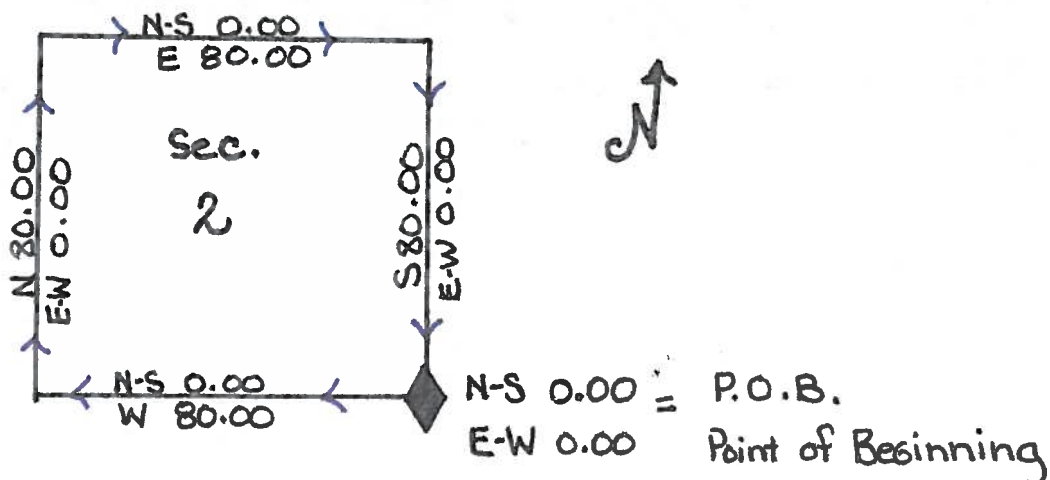
4. GO or MOVE

S 10.00 or S 75-57-50 W  
W 40.00 41.23

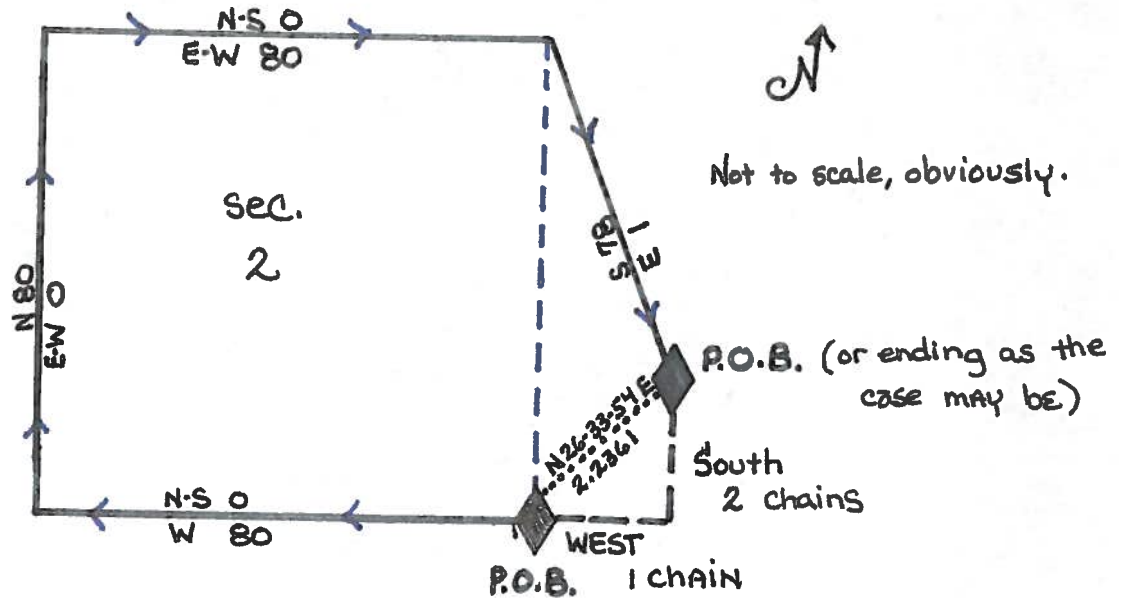


## MISCLOSURE, CLOSURE & ERROR

When a traverse begins + ends at the same point, as we just did in the sample traverse around section 8, it is called a CLOSED TRAVERSE. All traverses must be CLOSED in order to determine whether any error (human, electronic, mother nature ..... etc.) introduced into the survey is within acceptable limits.



The beginning Latitude and Departure for a typical traverse is N-S 0.00 + E-W 0.00, (We haven't gone anywhere yet!!). If we begin at the SE cor. of sec. 2 and traverse clockwise around it given the above LATS. + DEPS.; upon CLOSING BACK INTO the SE cor., we would be back to Lat. + Dep. of N-S 0.00 + E-W 0.00. This is called CLOSING FLAT, or no error or misclosure; and rarely, if ever, happens.

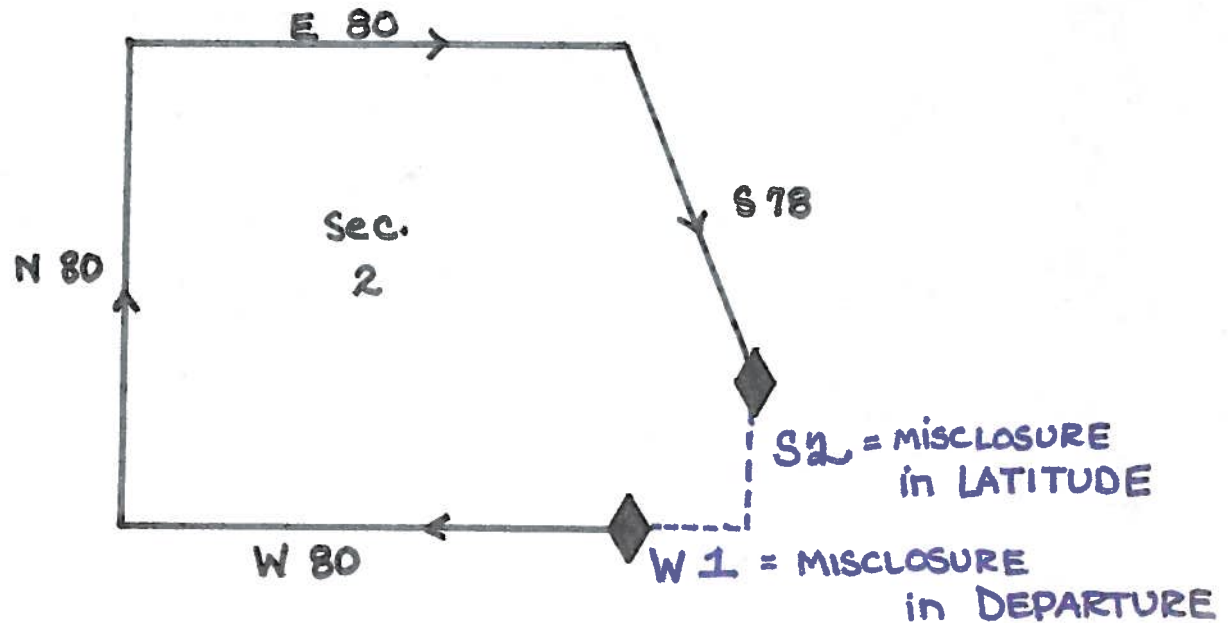


Let's say we begin the traverse at the SE COR. AGAIN, using the above Lats. + Deps. This time upon reaching the P.O.B., the TOTAL of the LATS. + Deps. does not equal zero, zero. The distance in each direction that we MISS the SE COR. is our **MISCLOSURE**. We would need to go South 2 chains, and west 1 chain to CLOSE FLAT. It is the same to SAY WE ARE 2 chains North and 1 chain EAST of the P.O.B.  $N 2.00 + E 1.00$  can be converted into the bearing + distance of the <sup>(LINEAR ERROR OF CLOSURE)</sup> misclosure, which is sometimes helpful in searching for the error if the error is too great, or OUT of our Acceptable limits.

How much is TOO much ERROR?



**CLOSURE** can be calculated by dividing the total distance surveyed around the perimeter of a closed traverse, by the misclosure in both the Lat. and Dep., individually.



Using the same Lats. + Deps. for Sec. 2, ADD together (All positive values for this) the total distances (in cardinal directions) travelled around sec. 2.

\*  $80 + 80 + 80 + 78 = 318$  CHAINS = Total horizontal distance around sec. 2. ( $\Sigma$  HD).

Divide the  $\Sigma$ HD by the misclosure in LATITUDE.

$$\frac{318}{2} = 159 \text{ chains.}$$

Divide the  $\Sigma$ HD by the misclosure in DEPARTURE.

$$\frac{318}{1} = 318 \text{ chains.}$$



## Limits of Closure

$$\frac{\sum \text{HD } 318}{\text{MISCLOSURE LAT. } 2} = 159 \text{ CH.}$$

$$\frac{\sum \text{HD } 318}{\text{MISCLOSURE DEP. } 1} = 318 \text{ CH.}$$

For Sec. 2 we would say that our closure in Latitude is  $\frac{1}{159}$ ; which means that for every 159 chains travelled, we were 1 chain off, or we would misclose by 1 chain. Likewise, the closure in Departure is  $\frac{1}{318}$ ; for every 318 chains travelled, we were 1 chain off. That is a bunch OF ERROR, and we'd say this section DID NOT CLOSE..... It would be necessary to go back in the field and find the problem. So - the LIMITS OF CLOSURE that we are required to return ARE:

→  $\frac{1}{5,000}$  on any closed survey, using all our own measurements.  
OR

→  $\frac{1}{2,500}$  if our survey uses a record measurement from a previously Approved survey.

In some instances a particular jobs' Special Instructions may set a different limit of closure depending on the circumstances.

Consequently, the  $\frac{1}{159}$  and the  $\frac{1}{318}$  aren't even close to the  $\frac{1}{5000}$  required. In a section with a perimeter of 320 chains, we'd need an error of closure, of less than  $6\frac{1}{2}$  Links to be within the  $\frac{1}{5000}$  Limit.

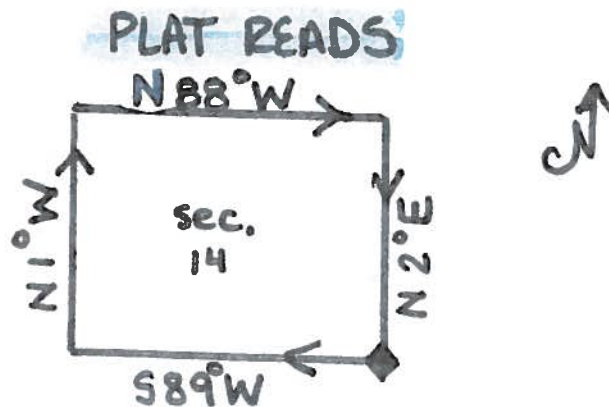
## Limits of Closure Practice

FOR EACH of the sections on the following PAGE, FIND:

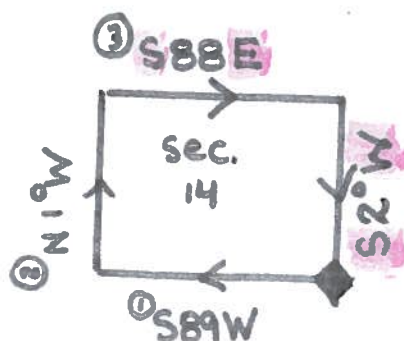
1. Misclosure in LAT.
2. Misclosure in DEP.
3. BEAR. + Dist. of the misclosure.
4. Closure in LAT.,  $\frac{1}{2}$  is it within limits.
5. Closure in DEP.,  $\frac{1}{2}$  is it within limits.

**NOTE:** On a plat, bearings are usually written in terms of Westing, on E-W lines; and Northing on N-S lines. Don't let it confuse you.... just begin at a corner and proceed clockwise around the section, as you would on the ground.

**EXAMPLE:**

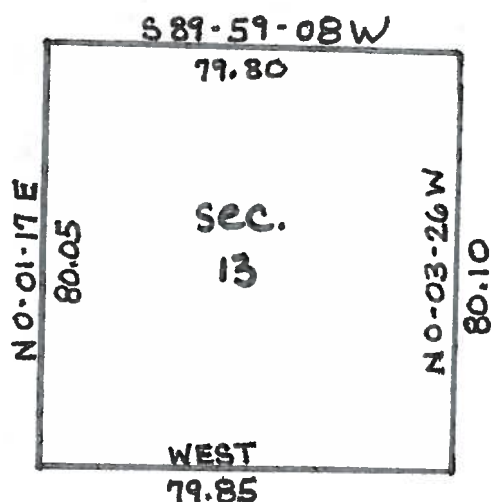


But you can think of it as:



## Limits of Closure Practice

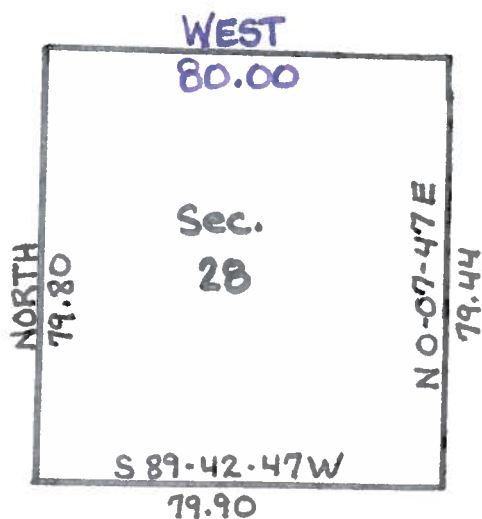
1. Sec. 13 was SURVEYED using OUR OWN MEASUREMENTS.



### FIND:

- |   |                      |
|---|----------------------|
| 1. Misclosure in LAT. ....                    | 50.03                |
| 2. Misclosure in Dep. ....                    | E 0.06               |
| 3. BEARING + DIST.<br>of the misclosure. .... | S 63-26-06 E<br>0.07 |
| 4. Closure in LAT. ....                       | 1/10,660             |
| Is it within limits? ...                      | YES                  |
| 5. Closure in Dep. ....                       | 1/5,330              |
| Is it within limits? ...                      | YES                  |

2. Sec. 28 was SURVEYED using Record bearing & distance for the North boundary.



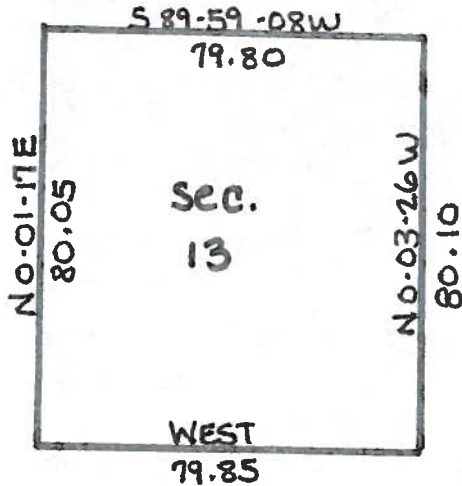
### FIND:

- |   |                      |
|---|----------------------|
| 1. Misclosure in Lat. ....                    | S 0.04               |
| 2. Misclosure in Dep. ....                    | W 0.08               |
| 3. BEARING + DIST.<br>of the misclosure. .... | S 63-26-06 W<br>0.09 |
| 4. Closure in LAT. ....                       | 1/7,979              |
| Is it within limits? ...                      | YES                  |
| 5. Closure in Dep. ....                       | 1/3,989              |
| Is it within limits? ...                      | YES                  |

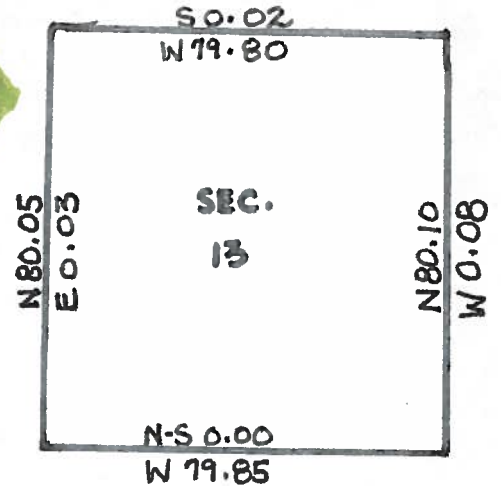


## Step by step SOLUTION to #1. - Limits of Closure

1. Sec. 13, surveyed using our own measurements.



STEP #1  
CHANGE BR. + DIST.  
TO LAT. + DEP.



STEP #2 - Find  $\Sigma HD$

$$\begin{array}{r} 79.85 \\ 80.05 \\ 79.80 \\ + 80.10 \\ \hline 319.80 = \Sigma HD \end{array}$$

STEP #9  
BR. + DIST. OF  
MISCLOSURE

$$\begin{array}{l} S 0.03 \uparrow E 0.06 = \\ S 63-26-06 E \\ 0.07 \end{array}$$

STEP #3 - Find Misclosure  
in LAT.

(I'm starting at the SE Cor.)

$$\begin{array}{r} 0.00 \\ 80.05 \\ 0.02 \\ + (-80.10) \\ \hline -0.03 \text{ or } S 0.03 \end{array}$$

STEP #4 - Closure in LAT.

$$\frac{319.80}{.03} = 10,660$$

STEP #5 - Find Misclosure in DEP.

$$\begin{array}{r} * (-79.85) \\ 0.03 \\ 79.80 \\ \hline 0.08 \\ 0.06 \text{ or } E 0.06 \end{array}$$

STEP #6 - Closure in DEP.

$$\frac{319.80}{0.06} = 5,330$$

STEP #7 - LAT. WITHIN ?  
LIMITS.

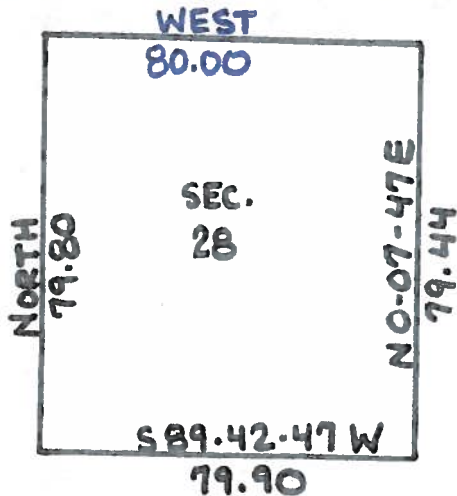
$\frac{1}{10,660}$  is better than  
 $\frac{1}{5,000}$  - YES.

STEP #8 - DEP. WITHIN ?  
LIMITS.

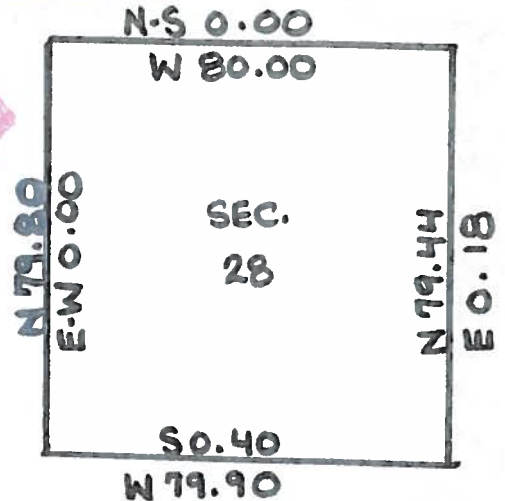
$\frac{1}{5,330}$  is better than  
 $\frac{1}{5,000}$  - YES.

## Step by Step Solution to #2 - Limits of Closure.

2. SEC. 28 used a record bearing + distance.



STEP #1  
CHANGE BR. + DIST.  
TO LAT. + DEP.



STEP #2 - Find ΣHD

$$\begin{array}{r} 79.90 \\ 79.80 \\ 80.00 \\ + 79.44 \\ \hline 319.14 = \Sigma HD \end{array}$$

STEP #3 - FIND MISCLOSURE  
in LAT.

$$\begin{array}{r} (-0.40) \\ 79.80 \\ 0.00 \\ + (-79.44) \\ \hline -0.04 \text{ or } S 0.04 \end{array}$$

STEP #4 - CLOSURE IN LAT.

$$\frac{319.14}{0.04} = 7,979$$

STEP #5 - FIND MISCLOSURE IN DEP.

$$\begin{array}{r} (-79.90) \\ 0.00 \\ 80.00 \\ \hline (-0.18) \\ -0.08 \text{ or } W 0.08 \end{array}$$

STEP #6 - CLOSURE IN DEP.

$$\frac{319.14}{0.08} = 3,989$$

STEP #7 - LAT. Within Limits?

$\frac{1}{7,979}$  is better than  
 $\frac{1}{2,500}$  - YES.

STEP #8 - DEP. Within Limits?

$\frac{1}{3,989}$  is better than  
 $\frac{1}{2,500}$  - YES.

STEP #9 - BR. + DIST. of Misclosure

S 0.04  $\frac{1}{2}$  W 0.08

S 63-26-06 W

0.09

## Appendix 2 - Standard Corner Numbering

Many of the proportion routines for rectangular corners in CMM are dependent upon or can perform more automated operations if the standard numbering system is used to identify the controlling or 'found' corners. Those of you familiar with the Bureau of Land Managements Geographic Coordinate Data Base (GCDB) Project will recognize similarities to their system. However the system in use within CMM is simplified and has some significant differences required to meet CMM requirements. *Some places in the documentation do refer to the system as the GCDB system.* Initially it was intended to make CMM independent of the corner identification system, as there are many effective rectangular corner identification systems that have been used throughout the years by different people and agencies. However the improved functionality that accrues in using a specific standard began to emerge as an important attribute. As a result at this point in the system development and use several programs make extensive use of the system:

**AUTOPROP** for example is dependent upon the system.

**INREC's** efficient record entry capabilities are only realised when using the standard numbering.

**PROPORT** is not dependent on the system however it is capable of making a better determination of the proper proportion method when the system is used.

**ADJUST** is independent of the system.

**WHATIS's** controlling corner tagging and **SECTSHOW's** control corner depiction algorithms are dependent upon the system.

For more details see the Reference section documentation for the above programs.

### The System

The system consists of a base 6 digit number. The first 3 digits, somewhat like an X coordinate represent corners on the N-S section lines, counting from the West boundary of a standard township. The second 3 digits, similar to a Y coordinate, represent corners located on the the E-W section lines counting from the south. Corners below the section corner level are indicated in approx. chain units i.e. counting from the West and South boundaries of the section. For example the 1/4 corner bet. 20 and 21 is: 300340.



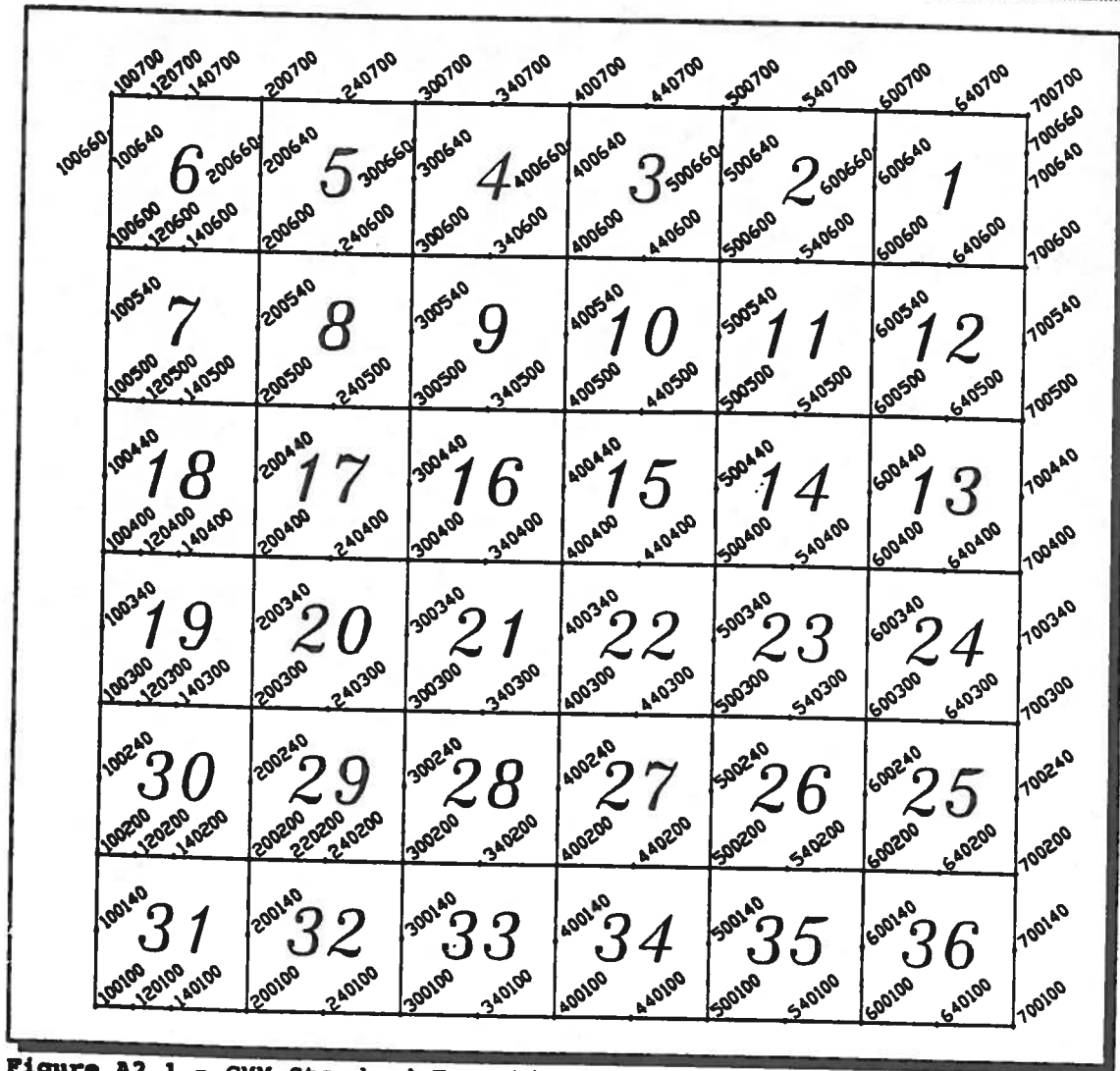


Figure A2.1 - CMM Standard Township Numbering

In the CMM standard system:

- \* Section corners are characterized by even x00y00 values, like 500300.
- \* Quarter Section corners are characterized by even 40 chain values like 500340, 540300 or 540340 which is the center 1/4 section corner of section 23.
- \* Sixteenth Section corners are characterized by even 20 chain values exclusive of the above. Values like 500320, 500360, 560300 (the East 1/16 corner between sections 23 and 26), and 520360 is the SE 1/16 corner of section 23.
- \* Corners below the 1/16 level are not necessary in the system, that is neither PROPORT or AUTOPROP compute corners at that level automatically.
- \* Creativity is required in extended sections. 1/16-80 corners can receive a 400680 designation, but sections elongated beyond 90 chains will require your imagination.

\* Witness corners, meander corners, closing corners, etc. are designated at their approximate chain value in the system being careful to avoid EVEN chain increments. For example: a closing corner, CC, on the north boundary of the township that is offset 20 chains from the standard corners might logically be labelled 420700, but since this would indicate a 'regular' 1/16 section corner it should not be used. Instead 419700 or 421700 would be possible selections.

This is a departure from the GCDB system in at least two ways:

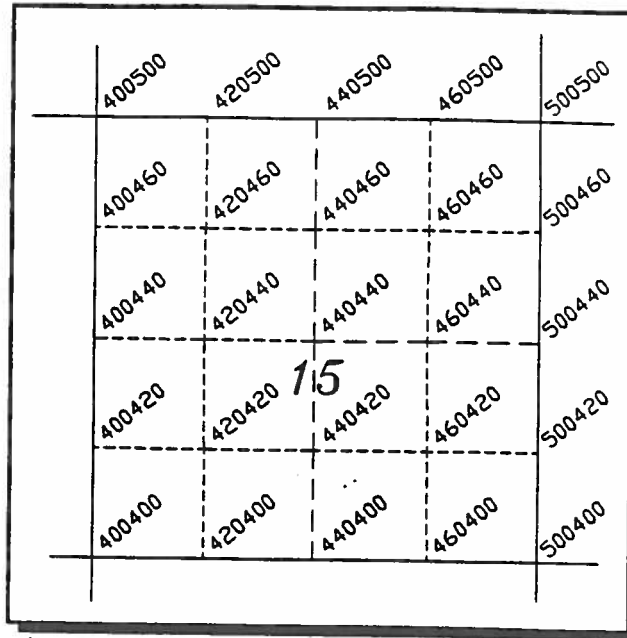


Figure A2.3 - Typical Subdivision Numbering

1) In CMM no particular rules are applied to these odd numbered corners, however GCDB rules that do not conflict with this system can be used.

2) Closing Corners and Standard Corners are designated according to their function in the PLSS rather than their role in the particular township. In GCDB the CC between sections 1 and 2 functions as a section corner and is designated 600700 for the township to the south, but it is not a section corner for the township to the north.

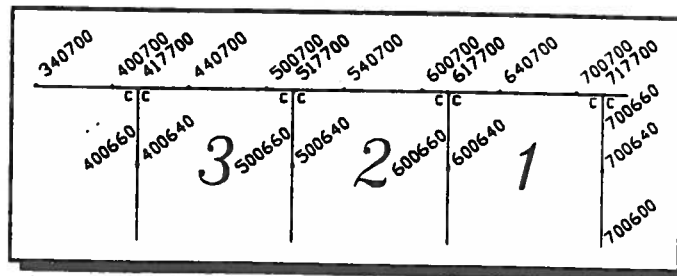


Figure A2.2 - Typical closing corner designation

In that township it would receive an 'odd' designation. In CMM this CC would receive the same first 3 digit designation for both townships i.e. 617700 for the south township and 617100 in the north township. In CMM the function of the corner is determined by connectivity in .REC file.

*The PLSS is very much more complex a system to be able to easily define rules to apply to all cases, user intuition and creativity will have to deal with unusual cases.*